# PROPOSED
# TOTAL MAXIMUM DAILY LOAD (TMDL)

**for**

**E. Coli**

**in the**

**Powell River Watershed**

**(HUC 06010206)**

**Campbell, Claiborne, Hancock, and Union Counties,**

**Tennessee**

**FINAL**

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## LIST OF ABBREVIATIONS

| | |
|---|---|
| ADB | Assessment Database |
| AFO | Animal Feeding Operation |
| BMP | Best Management Practices |
| BST | Bacteria Source Tracking |
| CAFO | Concentrated Animal Feeding Operation |
| CFR | Code of Federal Regulations |
| CFS | Cubic Feet per Second |
| CFU | Colony Forming Units |
| DEM | Digital Elevation Model |
| DWPC | Division of Water Pollution Control |
| E. coli | Escherichia coli |
| EPA | Environmental Protection Agency |
| GIS | Geographic Information System |
| HSPF | Hydrological Simulation Program - Fortran |
| HUC | Hydrologic Unit Code |
| LA | Load Allocation |
| LDC | Load Duration Curve |
| LSPC | Loading Simulation Program in $C^{++}$ |
| MGD | Million Gallons per Day |
| MOS | Margin of Safety |
| MRLC | Multi-Resolution Land Characteristic |
| MS4 | Municipal Separate Storm Sewer System |
| MST | Microbial Source Tracking |
| NHD | National Hydrography Dataset |
| NMP | Nutrient Management Plan |
| NPS | Nonpoint Source |
| NPDES | National Pollutant Discharge Elimination System |
| NRCS | Natural Resources Conservation Service |
| PCR | Polymerase Chain Reaction |
| PDFE | Percent of Days Flow Exceeded |
| PFGE | Pulsed Field Gel Electrophoresis |
| Rf3 | Reach File v.3 |
| RM | River Mile |
| SSO | Sanitary Sewer Overflow |
| STP | Sewage Treatment Plant |
| SWMP | Storm Water Management Program |
| TDA | Tennessee Department of Agriculture |
| TDEC | Tennessee Department of Environment & Conservation |
| TDOT | Tennessee Department of Transportation |
| TMDL | Total Maximum Daily Load |
| TWRA | Tennessee Wildlife Resources Agency |
| USGS | United States Geological Survey |
| UCF | Unit Conversion Factor |
| WCS | Watershed Characterization System |
| WLA | Waste Load Allocation |
| WWTF | Wastewater Treatment Facility |

## SUMMARY SHEET

### Total Maximum Daily Load for E. coli in Powell River Watershed (HUC 06010206)

Impaired Waterbody Information

State: Tennessee
Counties: Campbell, Claiborne, and Hancock
Watershed: Powell River (HUC 06010206)
Constituents of Concern: E. coli

Impaired Waterbodies Addressed in This Document:

| Waterbody ID | Waterbody | Miles Impaired |
|---|---|---|
| TN06010206006 – 0200 | GAP CREEK | 12.7 |
| TN06010206007 – 0100 | LITTLE CREEK | 9.4 |
| TN06010206007 – 0700 | MULBERRY CREEK | 26.6 |
| TN06010206007 – 0710 | LITTLE MULBERRY CREEK | 4.0 |
| TN06010206008 – 1000 | RUSSELL CREEK | 8.1 |
| TN06010206026 – 0100 | CAWOOD BRANCH | 5.2 |
| TN06010206026 – 0200 | RUSSELL BRANCH | 3.5 |
| TN06010206026 – 1000 | DAVIS CREEK | 8.0 |
| TN06010206026 – 2000 | DAVIS CREEK | 5.1 |
| TN06010206026 – 3000 | DAVIS CREEK | 3.6 |
| TN06010206026 – 4000 | DAVIS CREEK | 2.6 |

Designated Uses:

The designated use classifications for waterbodies in the Powell River Watershed include fish and aquatic life, irrigation, livestock watering & wildlife, and recreation.

Water Quality Targets:

Derived from *State of Tennessee Water Quality Standards, Chapter 1200-4-3, General Water Quality Criteria, 2007 Version* for recreation use classification (most stringent):

> The concentration of the E. coli group shall not exceed 126 colony forming units per 100 mL, as a geometric mean based on a minimum of 5 samples collected from a given sampling site over a period of not more than 30 consecutive days with individual samples being collected at intervals of not less than 12 hours. For the purposes of determining the geometric mean, individual samples having an E. coli concentration of less than 1 per 100 mL shall be considered as having a concentration of 1 per 100 mL.

Additionally, the concentration of the E. coli group in any individual sample taken from a lake, reservoir, State Scenic River, Exceptional Tennessee Water or ONRW (1200-4-3-.06) shall not exceed 487 colony forming units per 100 mL. The concentration of the E. coli group in any individual sample taken from any other waterbody shall not exceed 941 colony forming units per 100 mL.

*For further information on Tennessee's general water quality standards, see:*

http://www.state.tn.us/sos/rules/1200/1200-04/1200-04-03.pdf.

TMDL Scope:

Waterbodies identified on the Final 2008 303(d) list as impaired due to E. coli. TMDLs were developed for impaired waterbodies on a HUC-12 subwatershed or waterbody drainage area basis.

Analysis/Methodology:

The TMDLs for impaired waterbodies in the Powell River watershed were developed using a load duration curve methodology to assure compliance with the E. coli 126 CFU/100 mL geometric mean and the 487 CFU/100 mL maximum water quality criteria for lakes, reservoirs, State Scenic Rivers, or Exceptional Tennessee Waters and 941 CFU/100 mL maximum water quality criterion for all other waterbodies. A duration curve is a cumulative frequency graph that represents the percentage of time during which the value of a given parameter is equaled or exceeded. Load duration curves are developed from flow duration curves and can illustrate existing water quality conditions (as represented by loads calculated from monitoring data), how these conditions compare to desired targets, and the region of the waterbody flow zone represented by these existing loads. Load duration curves were also used to determine percent load reduction goals to meet the target maximum loading for E. coli. When sufficient data were available, load reductions were also determined based on geometric mean criterion.

Critical Conditions:

Water quality data collected over a period of up to 10 years for load duration curve analysis were used to assess the water quality standards representing a range of hydrologic and meteorological conditions.

For each impaired waterbody, critical conditions were determined by evaluating the percent load reduction goals, for each hydrologic flow zone, to meet the target (TMDL) loading for E. coli. The percent load reduction goal of the greatest magnitude corresponds with the critical flow zone.

Seasonal Variation:

The 10-year period used for LSPC model simulation period for development of load duration curve analysis included all seasons and a full range of flow and meteorological conditions.

Margin of Safety (MOS):

Explicit MOS = 10% of the E. coli water quality criteria for each impaired subwatershed or drainage area.

## Summary of TMDLs, WLAs, & LAs expressed as daily loads for Impaired Waterbodies in the Powell River Watershed (HUC 06010206)

| HUC-12 Subwatershed (06010206__) or Drainage Area (DA) | Impaired Waterbody Name | Impaired Waterbody ID | TMDL | MOS | WLAs | | | | LAs |
|---|---|---|---|---|---|---|---|---|---|
| | | | | | WWTFs [a] | Collection Systems | CAFOs | MS4s [b] | |
| | | | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/d/ac] | [CFU/d/ac] |
| 0205 | Little Mulberry Creek | TN06010104007 – 0710 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $7.877 \times 10^{6} \times Q$ |
| | Mulberry Creek | TN06010104007 – 0700 | $1.20 \times 10^{10} \times Q$ | $1.20 \times 10^{9} \times Q$ | NA | NA | NA | NA | $7.425 \times 10^{5} \times Q$ |
| 0301 (DA) | Little Creek | TN06010104007 – 0100 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $4.545 \times 10^{6} \times Q$ |
| 0302 | Russell Creek | TN06010104008 – 1000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | $2.315 \times 10^{10}$ | 0 | NA | NA | $1.913 \times 10^{6} \times Q - 2.140 \times 10^{6}$ |
| 0304 (DA) | Gap Creek | TN06010104006 – 0200 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | $1.843 \times 10^{9}$ | 0 | NA | NA | $2.170 \times 10^{6} \times Q - 1.933 \times 10^{5}$ |
| 0307 (DA) | Cawood Branch | TN06010104026 – 0100 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $9.668 \times 10^{6} \times Q$ |
| 0307 (DA) | Russell Branch | TN06010104026 – 0200 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $1.232 \times 10^{7} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 1000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $9.998 \times 10^{5} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 2000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $1.607 \times 10^{6} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 3000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $2.777 \times 10^{6} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 4000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $1.088 \times 10^{7} \times Q$ |

Notes: NA = Not Applicable.
Q = Mean Daily In-stream Flow (cfs).

a. WLAs for WWTFs are expressed as E. coli loads (CFU/day). All current and future WWTFs must meet water quality standards as specified in their NPDES permit.

b. Applies to any MS4 discharge loading in the subwatershed. Future MS4s will be assigned waste load allocations (WLAs) consistent with load allocations (LAs) assigned to precipitation induced nonpoint sources.

# PROPOSED E. COLI TOTAL MAXIMUM DAILY LOAD (TMDL) POWELL RIVER WATERSHED (HUC 06010206)

## 1.0 INTRODUCTION

Section 303(d) of the Clean Water Act requires each state to list those waters within its boundaries for which technology based effluent limitations are not stringent enough to protect any water quality standard applicable to such waters. Listed waters are prioritized with respect to designated use classifications and the severity of pollution. In accordance with this prioritization, states are required to develop Total Maximum Daily Loads (TMDLs) for those waterbodies that are not attaining water quality standards. State water quality standards consist of designated uses for individual waterbodies, appropriate numeric and narrative water quality criteria protective of the designated uses, and an antidegradation statement. The TMDL process establishes the maximum allowable loadings of pollutants for a waterbody that will allow the waterbody to maintain water quality standards. The TMDL may then be used to develop controls for reducing pollution from both point and nonpoint sources in order to restore and maintain the quality of water resources (USEPA, 1991).

## 2.0 SCOPE OF DOCUMENT

This document presents details of TMDL development for waterbodies in the Powell River Watershed, identified on the Final 2008 303(d) list as not supporting designated uses due to E. coli. TMDL analyses were performed primarily on a 12-digit hydrologic unit area (HUC-12) basis. In some cases, where appropriate, TMDLs were developed for an impaired waterbody drainage area only.

## 3.0 WATERSHED DESCRIPTION

The Powell River Watershed (HUC 06010206) is located in the northern portion of Eastern Tennessee (Figure 1), primarily in Campbell, Claiborne, and Hancock Counties. The Powell River Watershed lies within two Level III ecoregions (Ridge and Valley, Central Appalachians) and contains three Level IV ecoregions as shown in Figure 2 (USEPA, 1997):

- **The Southern Limestone/Dolomite Valleys and Low Rolling Hills (67f)** form a heterogeneous region composed predominantly of limestone and cherty dolomite. Landforms are mostly low rolling ridges and valleys, and the solids vary in their productivity. Landcover includes intensive agriculture, urban and industrial, or areas of thick forest. White oak forests, bottomland oak forests, and sycamore-ash-elm riparian forests are the common forest types, and grassland barrens intermixed with cedar-pine glades also occur here.
- **The Southern Sandstone Ridges (67h)** ecoregion encompasses the major sandstone ridges, but these ridges also have areas of shale and siltstone. The steep, forested chemistry of streams flowing down the ridges can vary greatly depending on the geologic material. The higher elevation ridges are in the north, including Wallen Ridge, Powell Mountain, Clinch Mountain, and Bays Mountain. White Oak Mountain in the south has some sandstone on the west side, but abundant shale and limestone as well. Grindstone Mountain, capped by the Gizzard Group sandstone, is the only remnant of

Pennsylvanian-age strata in the Ridge and Valley of Tennessee.

- The **Cumberland Mountains (69d)**, in contrast to the sandstone-dominated Cumberland Plateau (68a) to the west and southwest, are more highly dissected, with narrow-crested steep slopes, and younger Pennsylvanian-age shales, sandstones, siltstones, and coal. Narrow, winding valleys separate the mountain ridges, and relief is often 2000 feet. Cross Mountain, west of Lake City, reaches 3534 feet in elevation. Soils are generally well-drained, loamy, and acidic, with low fertility. The natural vegetation is a mixed mesophytic forest, although composition and abundance vary greatly depending on aspect, slope position, and degree of shading from adjacent land masses. Large tracts of land are owned by lumber and coal companies, and there are many areas of stripmining.

The Powell River Watershed, located in Campbell, Claiborne, Hancock, and Union Counties, Tennessee, has a drainage area of approximately 402 square miles ($mi^2$) in Tennessee. The entire watershed, including portions of Tennessee and Virginia, drains approximately 954 $mi^2$. Watershed land use distribution is based on the Multi-Resolution Land Characteristic (MRLC) databases derived from Landsat Thematic Mapper digital images from the period 1990-1993. Although changes in the land use of the Powell River Watershed have occurred since 1993 as a result of development, this is the most current land use data available. Land use in the Powell River Watershed is summarized in Table 1 and shown in Figure 3. Predominant land use in the Tennessee portion of the Powell River Watershed is forest (78.8%) followed by pasture (18.6%). Urban areas represent approximately 0.8% of the total drainage area of the watershed. Details of land use distribution of impaired subwatersheds in the Powell River Watershed are presented in Appendix A.

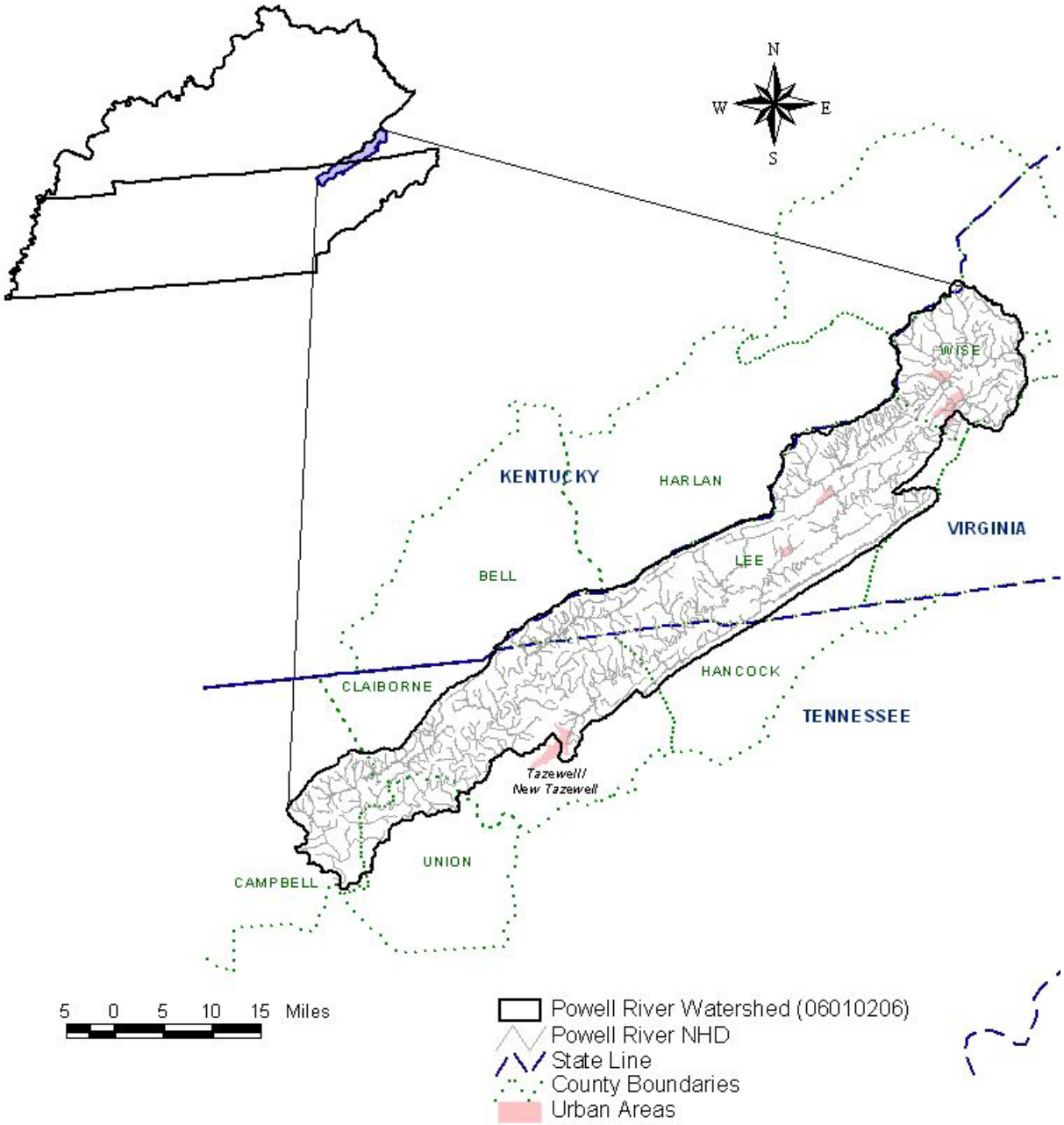


Figure 1. Location of the Powell River Watershed.

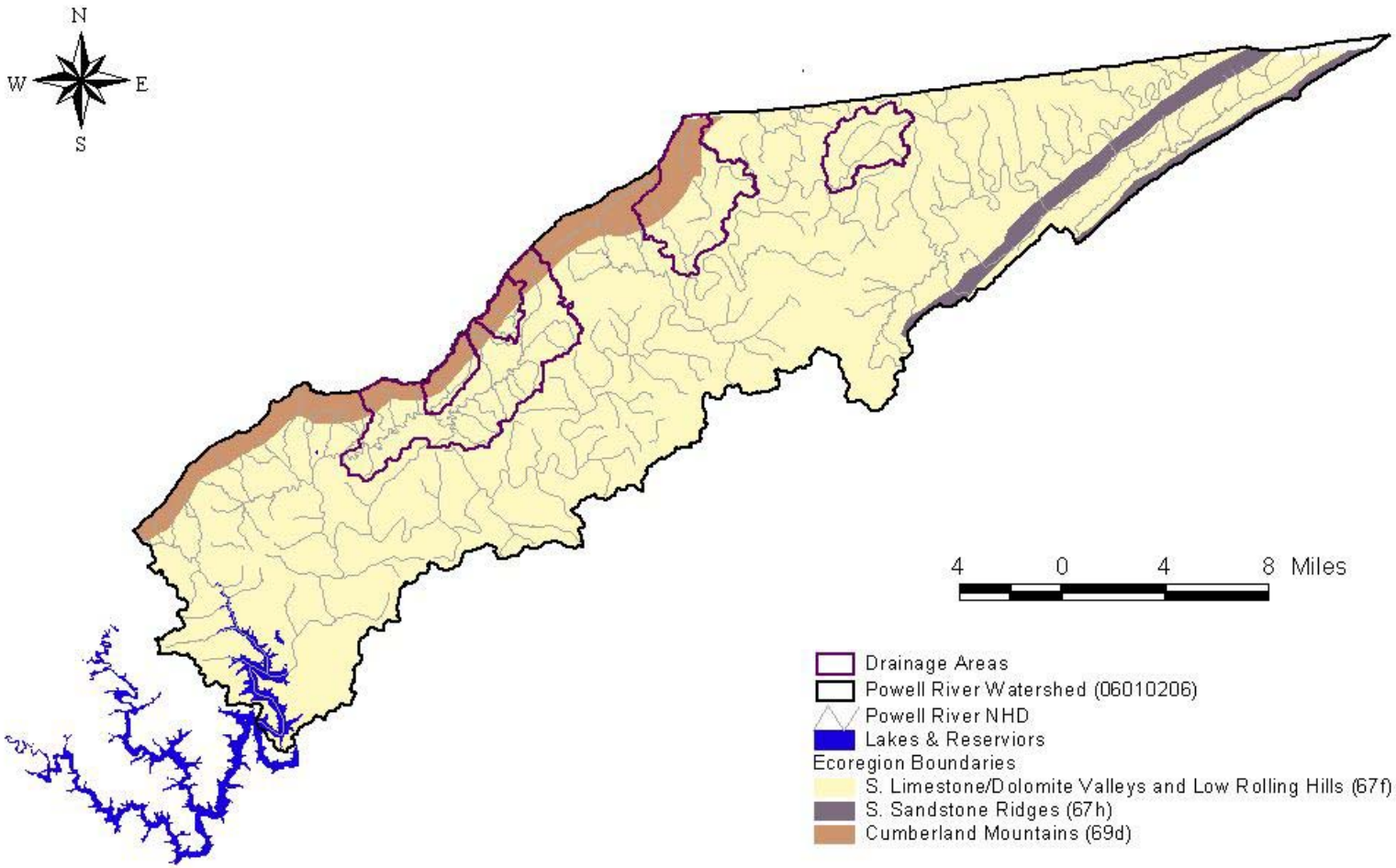


Figure 2. Level IV Ecoregions in the Powell River Watershed.

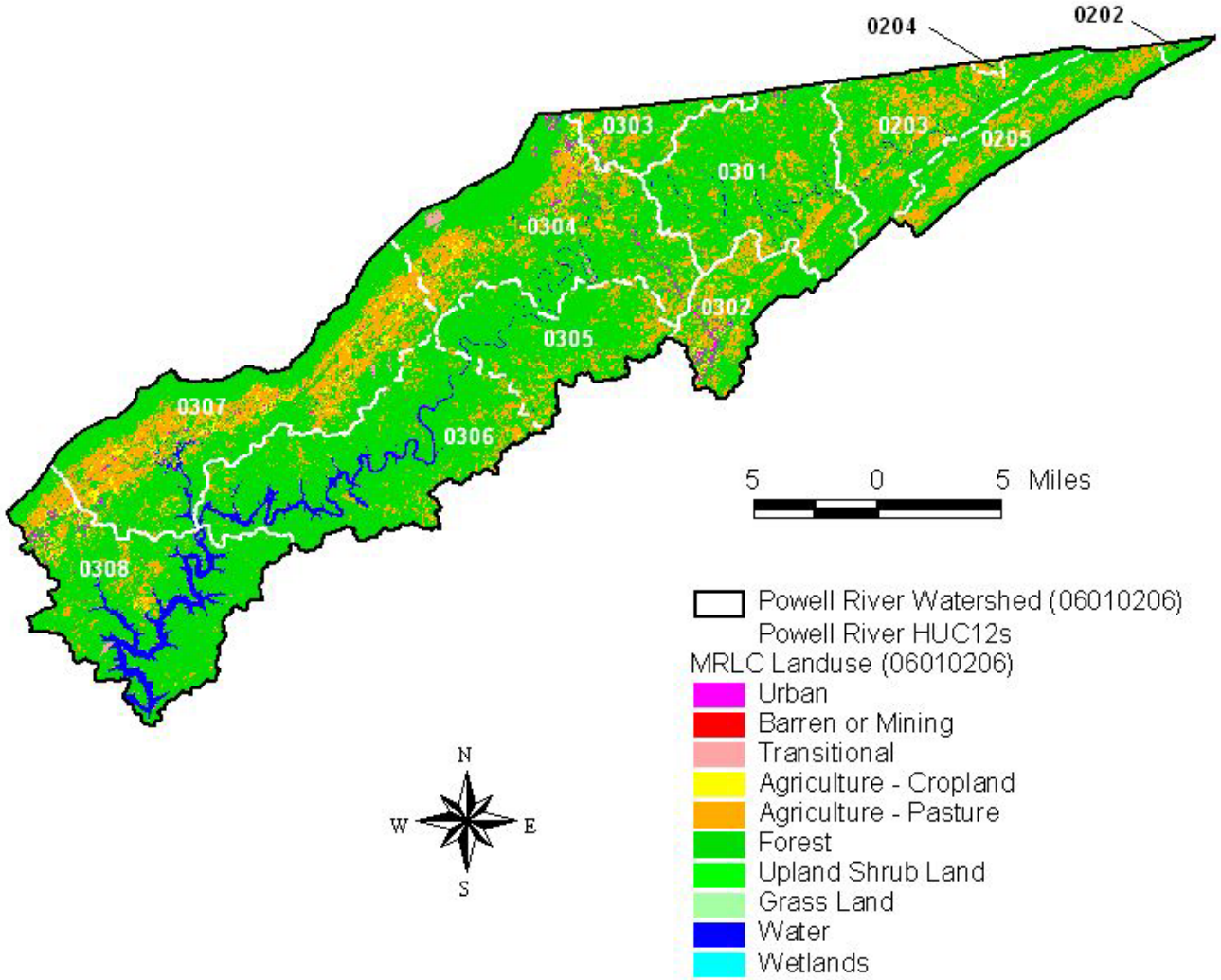


Figure 3. Land Use Characteristics of the Powell River Watershed.

**Table 1. MRLC Land Use Distribution – Powell River Watershed (Tennessee portion)**

| Land Use | Area | |
|---|---|---|
| | [acres] | [%] |
| Bare Rock/Sand/Clay | 14 | 0.0 |
| Deciduous Forest | 129,799 | 51.4 |
| Emergent Herbaceous Wetlands | 68 | 0.0 |
| Evergreen Forest | 22,491 | 8.9 |
| High Intensity Commercial/Industrial/ Transportation | 952 | 0.4 |
| High Intensity Residential | 90 | 0.0 |
| Low Intensity Residential | 966 | 0.4 |
| Mixed Forest | 36,746 | 14.5 |
| Open Water | 7,624 | 3.0 |
| Other Grasses (Urban/recreational) | 1,456 | 0.6 |
| Pasture/Hay | 47,030 | 18.6 |
| Quarries/Strip Mines/ Gravel Pits | 0 | 0.0 |
| Row Crops | 4,653 | 1.8 |
| Transitional | 879 | 0.3 |
| Woody Wetlands | 0 | 0.0 |
| **Total** | **252,768** | **100.0** |

## 4.0 PROBLEM DEFINITION

The State of Tennessee's final 2008 303(d) list (TDEC, 2008), http://state.tn.us/environment/wpc/publications/303d2008.pdf, was approved by the U.S. Environmental Protection Agency (EPA), Region IV in June of 2008. This list identified portions of 8 waterbodies in the Powell River Watershed as not fully supporting designated use classifications due, in part, to E. coli (see Table 2 & Figure 4). The designated use classifications for these waterbodies include fish and aquatic life, irrigation, livestock watering & wildlife, and recreation.

## 5.0 WATER QUALITY CRITERIA & TMDL TARGET

As previously stated, the designated use classifications for the Powell River waterbodies include fish & aquatic life, recreation, irrigation, and livestock watering & wildlife. Of the use classifications with numeric criteria for E. coli, the recreation use classification is the most stringent and will be used to establish target levels for TMDL development. The coliform water quality criteria, for protection of the recreation use classification, is established by *State of Tennessee Water Quality Standards, Chapter 1200-4-3, General Water Quality Criteria, 2007 Version* (TDEC, 2007).

Mulberry Creek has been classified as an Exceptional Tennessee Water due to the presence of several Federal endangered species. The portion of Gap Creek in the Cumberland Gap National Part has also been classified as an Exceptional Tennessee Water. As of August 6, 2007, none of the other impaired waterbodies in the Powell River Watershed have been classified as lakes, reservoirs, State Scenic Rivers, or Exceptional Tennessee Waters.

For further information concerning Tennessee's general water quality criteria and Tennessee's Antidegradation Statement, including the definition of Exceptional Tennessee Water, see:

http://www.state.tn.us/sos/rules/1200/1200-04/1200-04-03.pdf .

The geometric mean standard for the E. coli group of 126 colony forming units per 100 ml (CFU/100 ml) and the sample maximum of 487 CFU/100 ml have been selected as the appropriate numerical targets for TMDL development for impaired waterbodies classified as lakes, reservoirs, State Scenic Rivers, or Exceptional Tennessee Waters. The geometric mean standard for the E. coli group of 126 colony forming units per 100 ml (CFU/100 ml) and the sample maximum of 941 CFU/100 ml have been selected as the appropriate numerical targets for TMDL development for the other impaired waterbodies.

**Table 2 Final 2008 303(d) List for E. coli Impaired Waterbodies – Powell River Watershed**

| Waterbody ID | Impacted Waterbody | Miles/Acres Impaired | Cause (Pollutant) | Pollutant Source |
|---|---|---|---|---|
| TN06010206006 – 0200 | GAP CREEK | 12.7 | Escherichia coli | Septic Tanks<br>Collection System Failure |
| TN06010206007 – 0100 | LITTLE CREEK | 9.4 | Escherichia coli | Septic Tanks |
| TN06010206007 – 0700 | MULBERRY CREEK | 26.6 | Escherichia coli | Pasture Grazing |
| TN06010206007 – 0710 | LITTLE MULBERRY CREEK | 4.0 | Escherichia coli | Pasture Grazing |
| TN06010206008 – 1000 | RUSSELL CREEK | 8.1 | Nitrates<br>Phosphates<br>Escherichia coli | Discharges from MS4 Area [1]<br>Pasture Grazing |
| TN06010206026 – 0100 | CAWOOD BRANCH | 5.2 | Escherichia coli | Pasture Grazing |
| TN06010206026 – 0200 | RUSSELL BRANCH | 3.5 | Nitrate<br>Loss of biological integrity due to siltation<br>Habitat loss due to alteration in stream-side or littoral vegetative cover<br>Escherichia coli | Pasture Grazing |
| TN06010206026 – 1000 | DAVIS CREEK | 8.0 | Nutrients<br>Loss of biological integrity due to siltation<br>Escherichia coli | Confined Animal Feeding Operation (point and nonpoint) |
| TN06010206026 – 2000 | DAVIS CREEK | 5.1 | Nitrate<br>Loss of biological integrity due to siltation<br>Habitat loss due to alteration in stream-side or littoral vegetative cover<br>Escherichia coli | Pasture Grazing<br>Confined Animal Feeding Operation (nonpoint) |
| TN06010206026 – 3000 | DAVIS CREEK | 3.6 | Nitrate<br>Loss of biological integrity due to siltation<br>Escherichia coli | Pasture Grazing<br>Confined Animal Feeding Operation (nonpoint) |
| TN06010206026 – 4000 | DAVIS CREEK | 2.6 | Nutrients<br>Loss of biological integrity due to siltation<br>Escherichia coli | Pasture Grazing<br>Confined Animal Feeding Operation (nonpoint) |

[1] Excerpt from the Final 2008 303(d) List. The “MS4” referred to is Tazewell; however, Tazewell is not a NPDES regulated Municipal Separate Storm Sewer System.

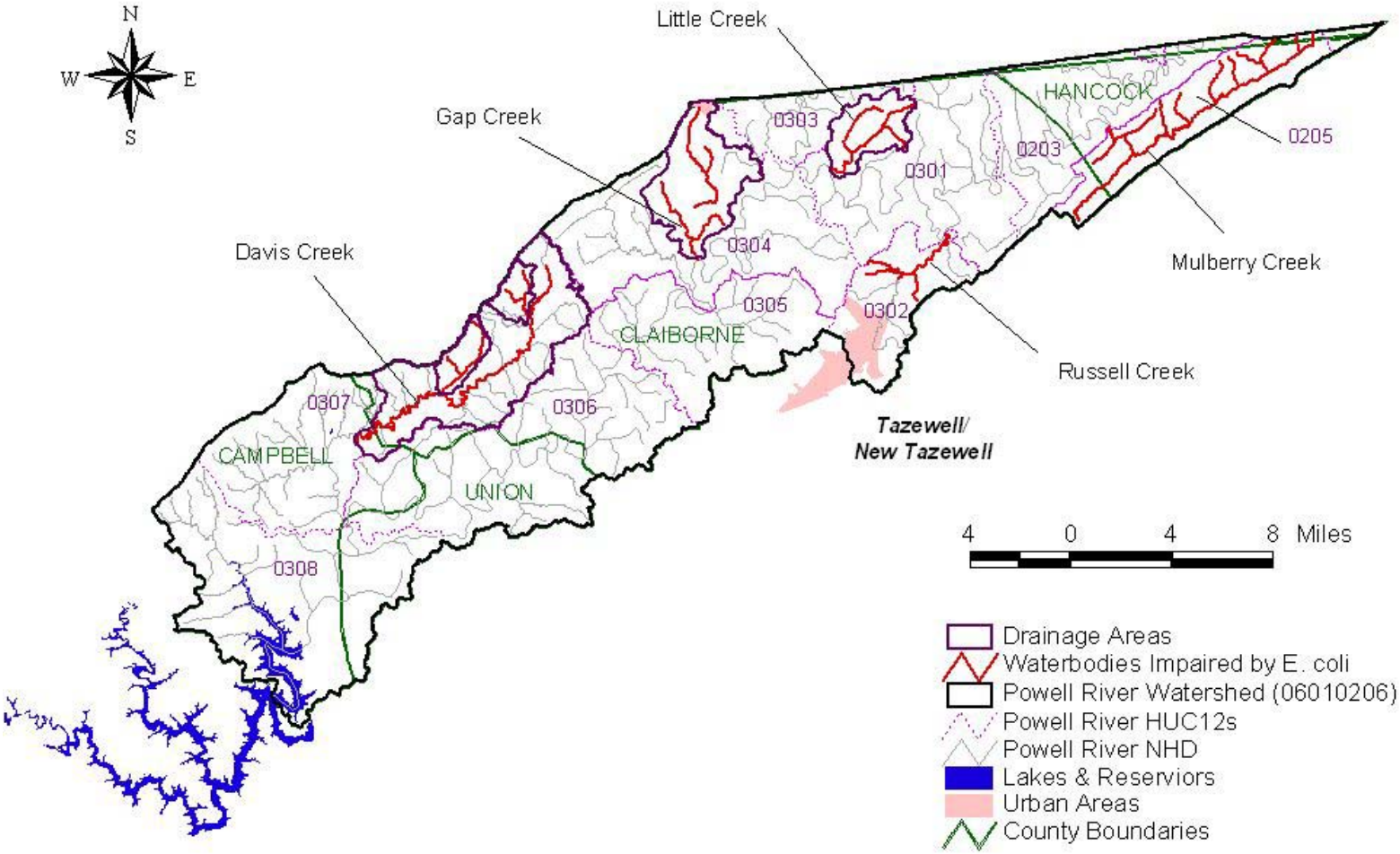


Figure 4. Waterbodies Impaired by E. Coli (as Documented on the Final 2008 303(d) List).

## 6.0 WATER QUALITY ASSESSMENT AND DEVIATION FROM TARGET

There are multiple water quality monitoring stations that provide data for waterbodies identified as impaired for E. coli in the Powell River Watershed. Monitoring stations located on lakes, reservoirs, State Scenic Rivers, or Exceptional Tennessee Waters have been italicized:

- HUC-12 06010206_0205:
  - LMULB000.6HK – Little Mulberry Creek, at Little Sycamore Rd.
  - *MULBE000.3HK – Mulberry Creek, at Hwy 63*
  - *MULBE000.7HK – Mulberry Creek, at mouth*
- HUC-12 06010206_0301:
  - LITTL000.1CL – Little Creek, at Little Creek Rd., near confluence Powell River
- HUC-12 06010206_0302:
  - RUSSE000.5CL – Russell Creek, about 0.1 mi d/s Colmon Br crossing
- HUC-12 06010206_0304:
  - GAP004.3CL – Gap Creek, at Tiprel Rd., u/s Hwy 63
- HUC-12 06010206_0307:
  - CAWOO000.2CL – Cawood Branch, 100 yds d/s Black Valley Rd.
  - DAVIS011.6CL – Davis Creek, d/s of Academy Rd bridge, at Speedwell Academy
  - DAVIS014.6CL – Davis Creek, u/s Ford at Old Hwy 63
  - DAVIS016.2CL – Davis Creek, at Russell Mill Rd.
  - DAVIS018.0/18.1CL – Davis Creek, at Davis Hensley Rd.
  - DAVIS020.5CL – Davis Creek, 100 yds u/s Singleton Rd. culvert
  - DAVIS022.6CL – Davis Creek, at Davis Lane, u/s confluence with Vanbebber Spring
  - RUSSE000.3CL – Russell Branch, 25 yds u/s Old Hwy 63

The locations of these monitoring stations are shown in Figure 5. Water quality monitoring results for these stations are tabulated in Appendix B. Examination of the data shows exceedances of the 487 CFU/100 mL (lakes, reservoirs, State Scenic Rivers, or Exceptional Tennessee Waters) and 941 CFU/100 mL (all other waterbodies) maximum E. coli standard at many monitoring stations. Water quality monitoring results for those stations with 10% or more of samples exceeding water quality maximum criteria are summarized in Table 3. Whenever a minimum of 5 samples was collected at a given monitoring station over a period of not more than 30 consecutive days, the geometric mean was calculated.

**Table 3 Summary of TDEC Water Quality Monitoring Data**

| Monitoring Station | Date Range | E. Coli (Max WQ Target = 941 CFU/100 mL)** | | | | |
|---|---|---|---|---|---|---|
| | | Data Pts. | Min. [CFU/100 ml] | Avg. [CFU/100 ml] | Max. [CFU/100 ml] | No. Exceed. WQ Max. Target |
| CAWOO000.2CL | 2000 | 11 | 166 | 885 | 2,419 | 4 |
| DAVIS011.6CL | 2000 | 12 | 39 | 574 | 2,419 | 2 |
| DAVIS014.6CL | 2000 – 2004 | 23 | 261 | 1,153 | 2,419 | 11 |
| DAVIS016.2CL | 2000 | 12 | 411 | 1,434 | 4,135 | 7 |
| DAVIS018.0CL | 2000 | 2 | 108 | 1,263 | 2,419 | 1 |
| DAVIS018.1CL | 2000 | 9 | 310 | 1,201 | 2,419 | 4 |
| DAVIS020.5CL | 2000 | 12 | 29 | 710 | 2,419 | 4 |
| DAVIS022.6CL | 2000 | 12 | 77 | 870 | 2,419 | 5 |
| GAP004.3CL | 2004 | 11 | 99 | 338 | 770 | 0 |
| LITTL000.1CL | 2002 – 2004 | 14 | 43 | 277 | 687 | 0 |
| LMULB000.6HK | 2000 – 2005 | 15 | 687 | 4,962 | 32,550 | 12 |
| *MULBE000.3HK* | *2002* | *3* | *365* | *1,081* | *2,419* | *1* |
| *MULBE000.7HK* | *2000 – 2005* | *15* | *68* | *2,227* | *20,460* | *10* |
| RUSSE000.3CL | 2000 | 12 | 260 | 2,092 | 6,086 | 11 |
| RUSSE000.5CL | 2004 | 12 | 157 | 493 | 1,733 | 1 |

** Maximum water quality target is 487 CFU/100 mL for lakes, reservoirs, State Scenic Rivers, or Exceptional Tennessee Waters waterbodies and 941 CFU/100 mL for other waterbodies. Waterbodies utilizing the 487 CFU/100 mL target are italicized.

Several of the water quality monitoring stations (Table 3 and Appendix B) have at least one E. coli sample value reported as >2419. For the purpose of calculating summary data statistics, TMDLs, Waste Load Allocations (WLAs), and Load Allocations (LAs), these data values are treated as (equal to) 2419. Therefore, the calculated results are considered to be estimates. Future E. coli sample analyses at these sites should follow established protocol. See Section 9.4.

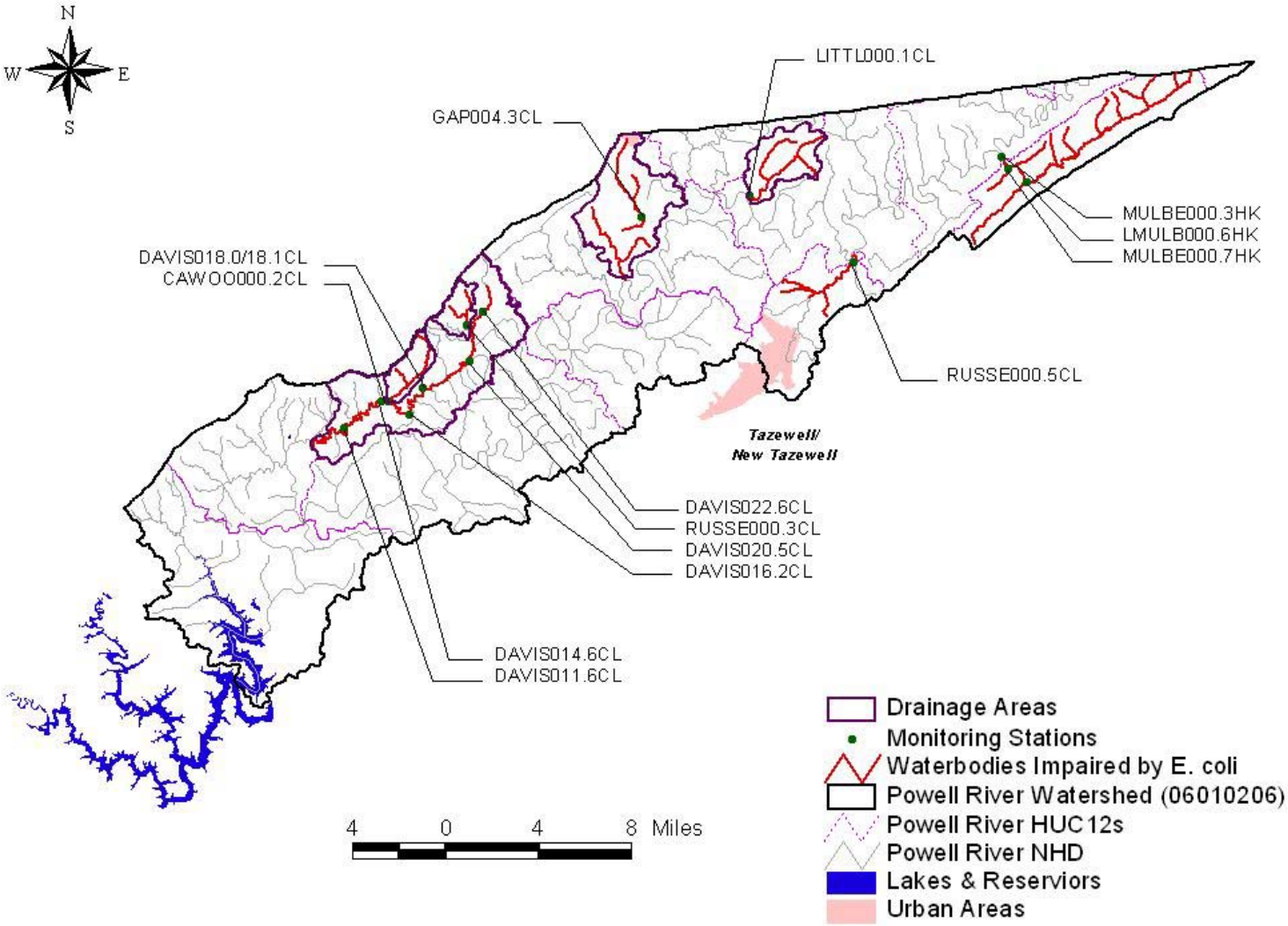


Figure 5. Water Quality Monitoring Stations in the Powell River Watershed

## 7.0 SOURCE ASSESSMENT

An important part of TMDL analysis is the identification of individual sources, or source categories of pollutants in the watershed that affect pathogen loading and the amount of loading contributed by each of these sources.

Under the Clean Water Act, sources are classified as either point or nonpoint sources. Under 40 CFR §122.2, (http://www.epa.gov/epacfr40/chapt-I.info/chi-toc.htm), a point source is defined as a discernable, confined, and discrete conveyance from which pollutants are or may be discharged to surface waters. The National Pollutant Discharge Elimination System (NPDES) program (http://cfpub1.epa.gov/npdes/index.cfm ) regulates point source discharges. Point sources can be described by three broad categories: 1) NPDES regulated municipal (http://cfpub1.epa.gov/npdes/home.cfm?program_id=13 ) and industrial (http://cfpub1.epa.gov/npdes/home.dfm?program_id=14 ) wastewater treatment facilities (WWTFs); 2) NPDES regulated industrial and municipal storm water discharges (http://cfpub1.epa.gov/npdes/home.cfm?program_id=6 ); and 3) NPDES regulated Concentrated Animal Feeding Operations (CAFOs) (http://cfpub1.epa.gov/npdes/home.cfm?program_id=7) ). A TMDL must provide Waste Load Allocations (WLAs) for all NPDES regulated point sources. Nonpoint sources are diffuse sources that cannot be identified as entering a waterbody through a discrete conveyance at a single location. For the purposes of this TMDL, all sources of pollutant loading not regulated by NPDES permits are considered nonpoint sources. The TMDL must provide a Load Allocation (LA) for these sources.

### 7.1 Point Sources

#### 7.1.1 NPDES Regulated Municipal and Industrial Wastewater Treatment Facilities

Both treated and untreated sanitary wastewater contain coliform bacteria. There are 6 WWTFs in the Powell River Watershed that have NPDES permits authorizing the discharge of treated sanitary wastewater. Four of these facilities are located in Tennessee and are also located in or near impaired subwatersheds or drainage areas (see Table 4 & Figure 6). However, only two of these facilities discharge to impaired waterbodies. The permit limits for discharges from these WWTFs are in accordance with the coliform criteria specified in Tennessee Water Quality Standards for the protection of the recreation use classification.

Non-permitted point sources of (potential) E. coli contamination of surface waters associated with STP collection systems include leaking collection systems (LCSs) and sanitary sewer overflows (SSOs).

> *Note: As stated in Section 5.0, the current coliform criteria are expressed in terms of E. coli concentration, whereas previous criteria were expressed in terms of fecal coliform and E. coli concentration. Due to differences in permit issuance dates, some permits still have fecal coliform limits instead of E. coli. As permits are reissued, limits for fecal coliform will be replaced by E. coli limits.*

**Table 4 NPDES Permitted WWTFs with Collection Systems Serving Impaired Subwatersheds or Drainage Areas**

| NPDES Permit No. | Facility | Design Flow [MGD] | Receiving Stream |
|---|---|---|---|
| TN0023256 | Lincoln Memorial University | 0.1 | Sinkhole |
| TN0024791 | Claiborne County Utility District | 0.65 | Russell Creek @RM6.0 |
| TN0059978 | Powell Valley Elementary School | 0.03 | Sinkhole |
| TN0060747 | Cumberland Gap STP | 0.1 | Gap Creek @RM8.7 |

7.1.2 NPDES Regulated Municipal Separate Storm Sewer Systems (MS4s)

Municipal Separate Storm Sewer Systems (MS4s) are considered to be point sources of E. coli. Discharges from MS4s occur in response to storm events through road drainage systems, curb and gutter systems, ditches, and storm drains. Phase I of the EPA storm water program (http://cfpub.epa.gov/npdes/stormwater/swphases.cfm#phase1 ) requires large and medium MS4s to obtain NPDES storm water permits. Large and medium MS4s are those located in incorporated places or counties serving populations greater than 100,000 people. At present, there are no MS4s of this size in the Powell River Watershed.

As of March 2003, regulated small MS4s in Tennessee must also obtain NPDES permits in accordance with the Phase II storm water program (http://cfpub.epa.gov/npdes/stormwater/swphases.cfm#phase2 ). A small MS4 is designated as *regulated* if: a) it is located within the boundaries of a defined urbanized area that has a residential population of at least 50,000 people and an overall population density of 1,000 people per square mile; b) it is located outside of an urbanized area but within a jurisdiction with a population of at least 10,000 people, a population density of 1,000 people per square mile, and has the potential to cause an adverse impact on water quality; or c) it is located outside of an urbanized area but contributes substantially to the pollutant loadings of a physically interconnected MS4 regulated by the NPDES storm water program. Most regulated small MS4s in Tennessee obtain coverage under the *NPDES General Permit for Discharges from Small Municipal Separate Storm Sewer Systems* (http://state.tn.us/environment/wpc/ppo/TN%20Small%20MS4%20Modified%20General%20Permit%202003.pdf ) (TDEC, 2003). None of the counties and municipalities in the Powell River Watershed are covered under Phase II of the NPDES Storm Water Program.

The Tennessee Department of Transportation (TDOT) has been issued an individual MS4 permit (TNS077585) that authorizes discharges of storm water runoff from State roads and interstate highway right-of-ways that TDOT owns or maintains, discharges of storm water runoff from TDOT owned or operated facilities, and certain specified non-storm water discharges. This permit covers all eligible TDOT discharges statewide, including those located outside of urbanized areas. TDOT's individual MS4 permit may be obtained from the Tennessee Department of Environment and Conservation (TDEC) website: http://state.tn.us/environment/wpc/stormh2o/TNS077585.pdf .

For information regarding storm water permitting in Tennessee, see the TDEC website:

http://www.state.tn.us/environment/wpc/stormh2o/.

### 7.1.3 NPDES Concentrated Animal Feeding Operations (CAFOs)

Animal feeding operations (AFOs) are agricultural enterprises where animals are kept and raised in confined situations. AFOs congregate animals, feed, manure and urine, dead animals, and production operations on a small land area. Feed is brought to the animals rather than the animals grazing or otherwise seeking feed in pastures, fields, or on rangeland (USEPA, 2002a). Concentrated Animal Feeding Operations (CAFOs) are AFOs that meet certain criteria with respect to animal type, number of animals, and type of manure management system. CAFOs are considered to be potential point sources of pathogen loading and are required to obtain an NPDES permit. Most CAFOs in Tennessee obtain coverage under TNA000000, *Class II Concentrated Animal Feeding Operation General Permit* (http://state.tn.us/environment/wpc/programs/cafo/CAFO_GP_04.pdf), while larger, Class I CAFOs are required to obtain an individual NPDES permit.

As of July 1, 2008, there are no Class I CAFOs with individual permits or Class II CAFOs with coverage under the general NPDES permit located in the Powell River Watershed. There is one Class II CAFO with pending coverage under the general NPDES permit. Hickory Corner Dairy (TNA000006) is located in the Davis Creek subwatershed. They have submitted a Nutrient Management Plan to the UT Extension office for review. Another AFO (Jennings Poultry – TNA000007), located in the Old Town Creek subwatershed, was covered under the previous general permit, but is not required to have coverage under the 2004 general permit.

## 7.2 Nonpoint Sources

Nonpoint sources of coliform bacteria are diffuse sources that cannot be identified as entering a waterbody through a discrete conveyance at a single location. These sources generally, but not always, involve accumulation of coliform bacteria on land surfaces and wash off as a result of storm events. Nonpoint sources of E. coli loading are primarily associated with agricultural and urban land uses. The vast majority of waterbodies identified on the Final 2008 303(d) List as impaired due to E. coli are attributed to nonpoint agricultural or urban sources.

### 7.2.1 Wildlife

Wildlife deposit coliform bacteria, with their feces, onto land surfaces where it can be transported during storm events to nearby streams. The overall deer density for Tennessee was estimated by the Tennessee Wildlife Resources Agency (TWRA) to be 23 animals per square mile.

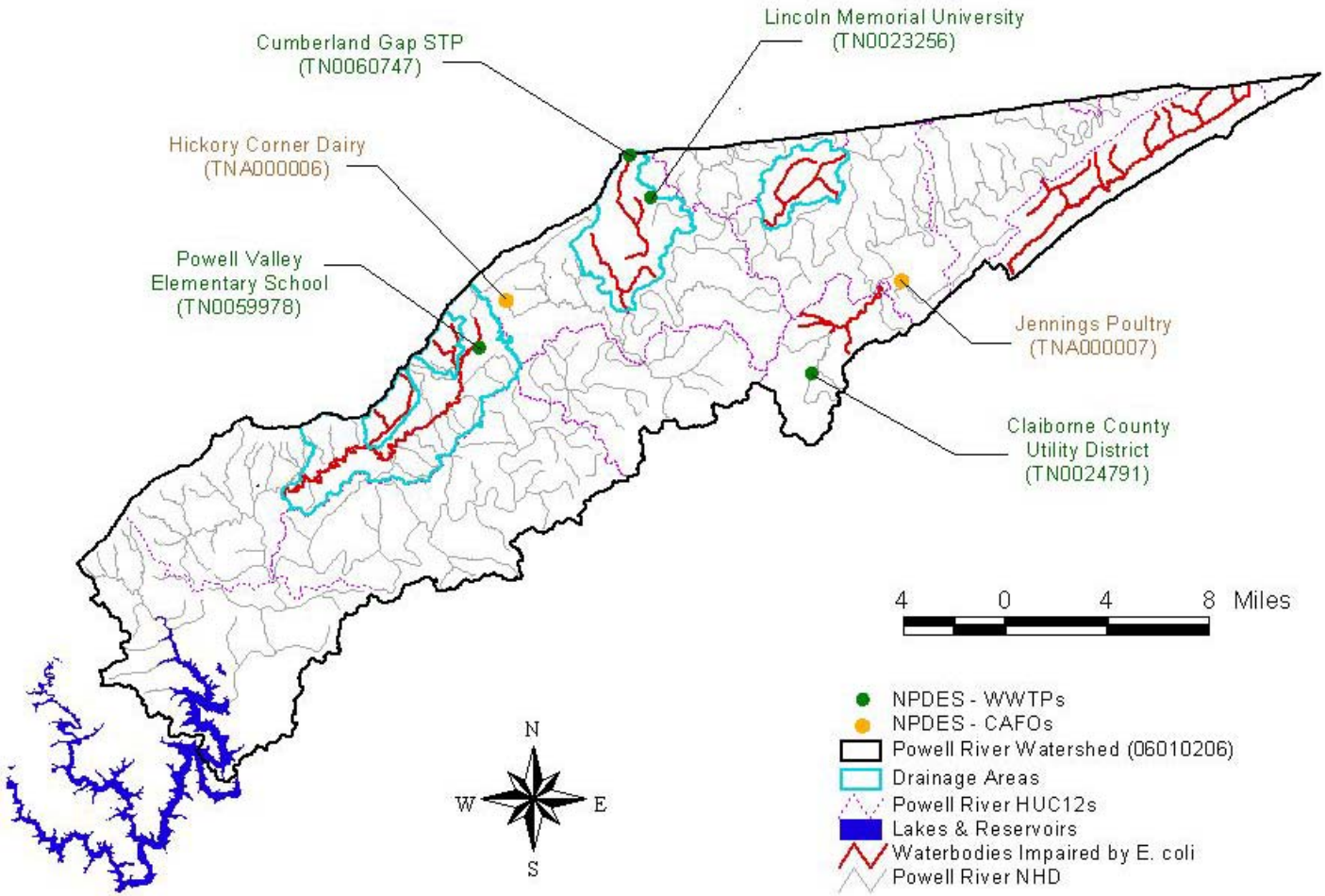


Figure 6. NPDES Regulated Point Sources in and near Impaired Subwatersheds and Drainage Areas of the Powell River Watershed.

### 7.2.2 Agricultural Animals

Agricultural activities can be a significant source of coliform bacteria loading to surface waters. The activities of greatest concern are typically those associated with livestock operations:

- Agricultural livestock grazing in pastures deposit manure containing coliform bacteria onto land surfaces. This material accumulates during periods of dry weather and is available for washoff and transport to surface waters during storm events. The number of animals in pasture and the time spent grazing are important factors in determining the loading contribution.
- Processed agricultural manure from confined feeding operations is often applied to land surfaces and can provide a significant source of coliform bacteria loading. Guidance for issues relating to manure application is available through the University of Tennessee Agricultural Extension Service and the Natural Resources Conservation Service (NRCS).
- Agricultural livestock and other unconfined animals often have direct access to waterbodies and can provide a concentrated source of coliform bacteria loading directly to a stream.

Data sources related to livestock operations include the 2002 Census of Agriculture (http://www.nass.usda.gov/census/census02/volume1/tn/index2.htm ). Livestock data for counties located within the Powell River watershed are summarized in Table 5. Note that, due to confidentiality issues, any tabulated item that identifies data reported by a respondent or allows a respondent's data to be accurately estimated or derived is suppressed and coded with a 'D' (USDA, 2004).

**Table 5 Livestock Distribution in the Powell River Watershed**

| County | Livestock Population (2002 Census of Agriculture) | | | | | | |
|---|---|---|---|---|---|---|---|
| | Beef Cow | Milk Cow | Poultry | | Hogs | Sheep | Horse |
| | | | Layers | Broilers | | | |
| Campbell | 4,575 | 115 | 386 | D | 53 | 39 | 444 |
| Claiborne | 17,868 | 689 | 409 | 111 | 123 | 104 | 978 |
| Hancock | 7,571 | 19 | 474 | D | 82 | 62 | 354 |
| Union | 5,928 | 66 | 828 | 90 | 46 | 27 | 798 |

* In keeping with the provisions of Title 7 of the United States Code, no data are published in the 2002 Census of Agriculture that would disclose information about the operations of an individual farm or ranch. Any tabulated item that identifies data reported by a respondent or allows a respondent's data to be accurately estimated or derived is suppressed and coded with a 'D' (USDA, 2004).

### 7.2.3 Failing Septic Systems

Some of the coliform loading in the Powell River watershed can be attributed to failure of septic systems and illicit discharges of raw sewage. Estimates from 1997 county census data of people in the Powell River watershed utilizing septic systems were compiled using the WCS and are summarized in Table 6. In middle and eastern Tennessee, it is estimated that there are approximately 2.37 people per household on septic systems, some of which can be reasonably assumed to be failing. As with livestock in streams, discharges of raw sewage provide a concentrated source of coliform bacteria directly to waterbodies.

**Table 6 Estimated Population on Septic Systems in the Powell River Watershed**

| County | Total Population (2000 Census) | Population on Septic Systems |
|---|---|---|
| Campbell | 39,854 | 32,752 |
| Claiborne | 29,862 | 26,137 |
| Hancock | 6,786 | 0 |
| Union | 17,808 | 13,303 |

### 7.2.4 Urban Development

Nonpoint source loading of coliform bacteria from urban land use areas is attributable to multiple sources. These include: stormwater runoff, illicit discharges of sanitary waste, runoff from improper disposal of waste materials, leaking septic systems, and domestic animals. Impervious surfaces in urban areas allow runoff to be conveyed to streams quickly, without interaction with soils and groundwater. Urban land use area in impaired subwatersheds in the Powell River Watershed ranges from 0.1% to 3.5%. Land use for the Powell River impaired drainage areas is summarized in Figures 7 thru 10 and tabulated in Appendix A.

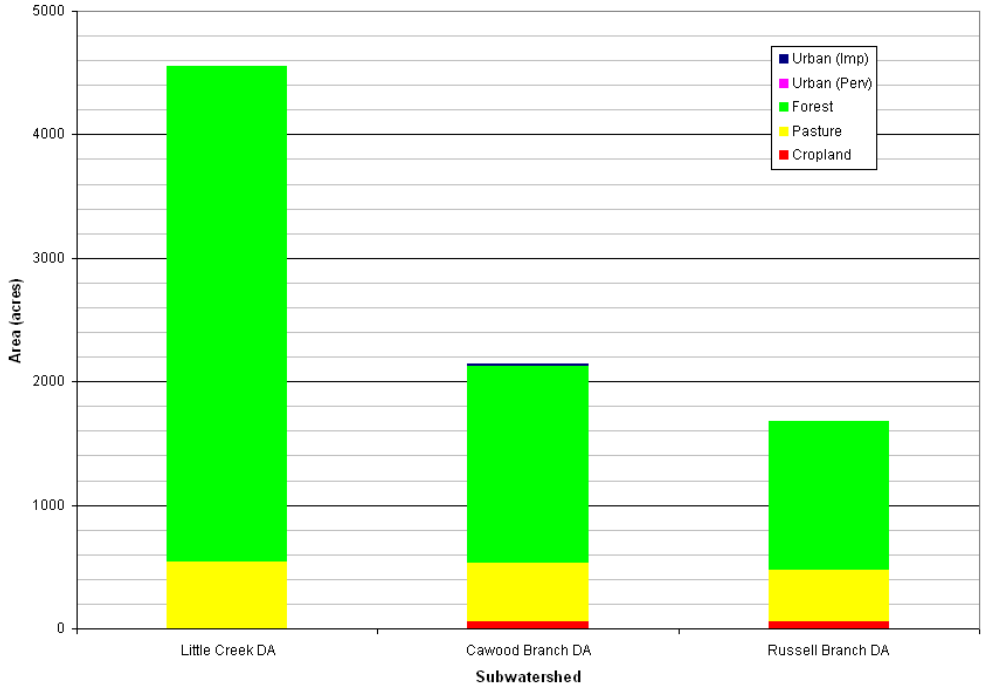


**Figure 7. Land Use Area of Powell River E. coli-Impaired Subwatersheds – Drainage Areas Less Than 5,000 Acres**

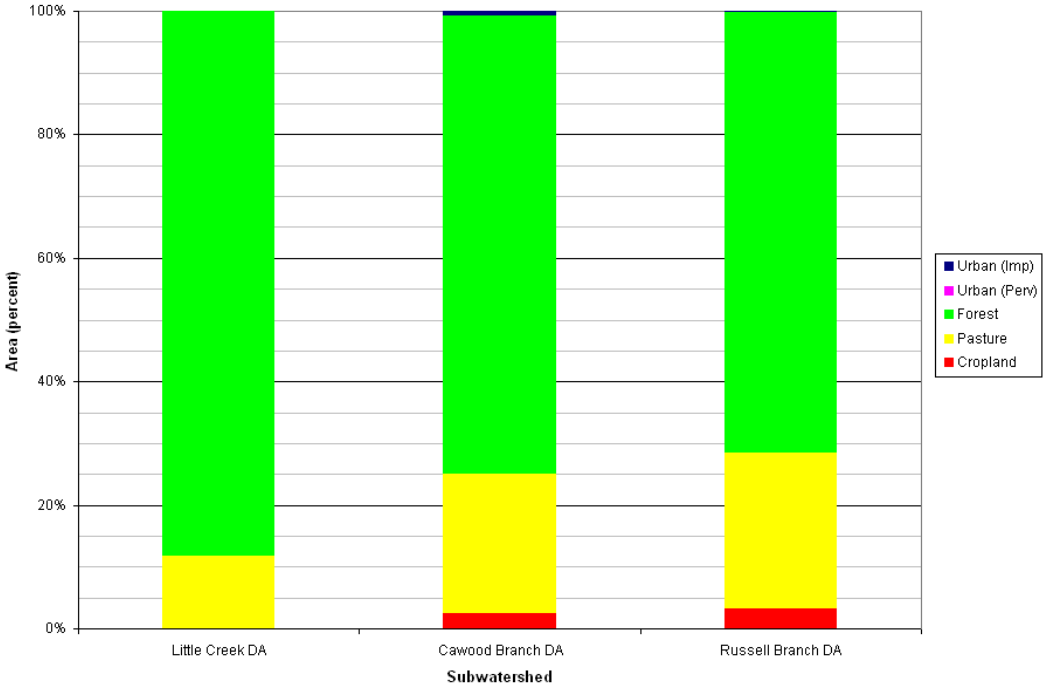


**Figure 8. Land Use Percent of the Powell River E. coli-Impaired Subwatersheds – Drainage Areas Less Than 5,000 Acres**

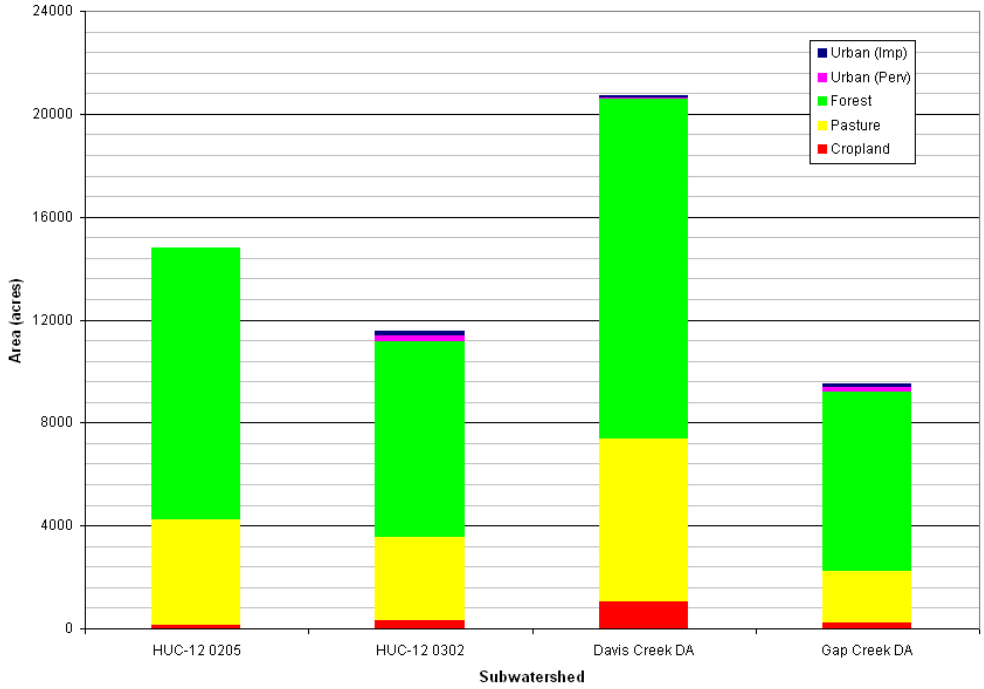


**Figure 9. Land Use Area of Powell River E. coli-Impaired Subwatersheds – Drainage Areas Greater Than 5,000 Acres**

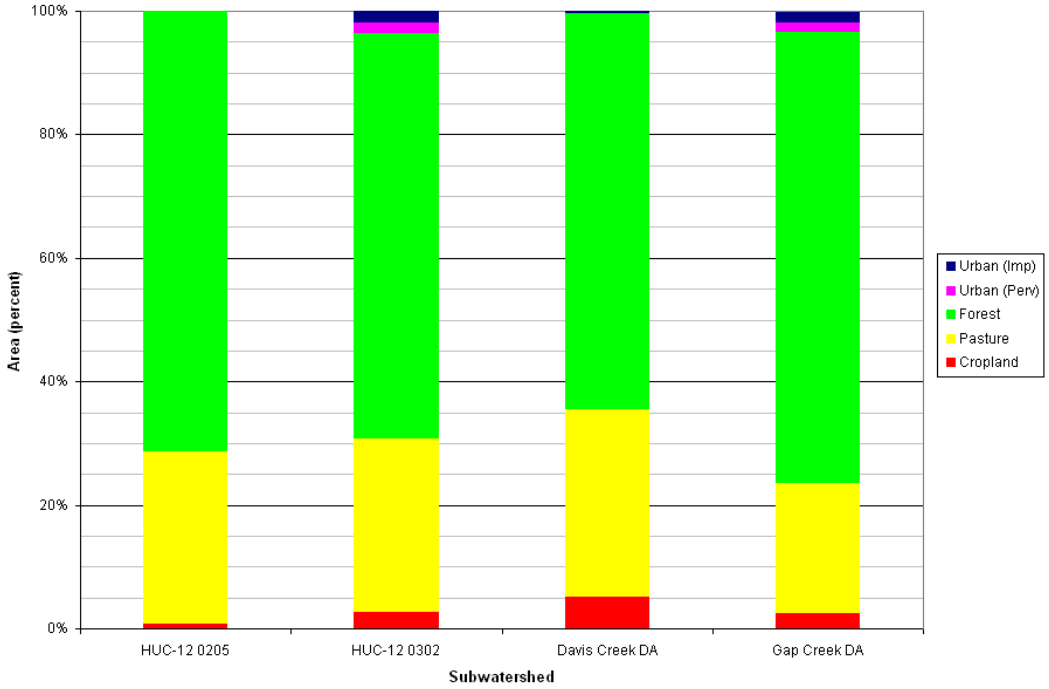


**Figure 10. Land Use Percent of the Powell River E. coli-Impaired Subwatersheds – Drainage Areas Greater Than 5,000 Acres**

## 8.0 DEVELOPMENT OF TOTAL MAXIMUM DAILY LOADS

The Total Maximum Daily Load (TMDL) process quantifies the amount of a pollutant that can be assimilated in a waterbody, identifies the sources of the pollutant, and recommends regulatory or other actions to be taken to achieve compliance with applicable water quality standards based on the relationship between pollution sources and in-stream water quality conditions. A TMDL can be expressed as the sum of all point source loads (Waste Load Allocations), non-point source loads (Load Allocations), and an appropriate margin of safety (MOS) that takes into account any uncertainty concerning the relationship between effluent limitations and water quality:

$$TMDL = \Sigma\ WLAs + \Sigma\ LAs + MOS$$

The objective of a TMDL is to allocate loads among all of the known pollutant sources throughout a watershed so that appropriate control measures can be implemented and water quality standards achieved. 40 CFR §130.2 (i) (http://www.epa.gov/epacfr40/chapt-I.info/chi-toc.htm ) states that TMDLs can be expressed in terms of mass per time, toxicity, or other appropriate measure.

This document describes TMDL, Waste Load Allocation (WLA), Load Allocation (LA), and Margin of Safety (MOS) development for waterbodies identified as impaired due to E. coli on the Final 2008 303(d) list.

### 8.1 Expression of TMDLs, WLAs, & LAs

In this document, the E. coli TMDL is a daily load expressed as a function of mean daily flow (daily loading function). For implementation purposes, corresponding percent load reduction goals (PLRGs) to decrease E. coli loads to TMDL target levels, within each respective flow zone, are also expressed. WLAs & LAs for precipitation-induced loading sources are also expressed as daily loading functions in CFU/day/acre. Allocations for loading that is independent of precipitation (WLAs for WWTFs and LAs for “other direct sources”) are expressed as CFU/day.

### 8.2 Area Basis for TMDL Analysis

The primary area unit of analysis for TMDL development was the HUC-12 subwatershed containing one or more waterbodies assessed as impaired due to E. coli (as documented on the Final 2008 303(d) List). In some cases, however, TMDLs were developed for an impaired waterbody drainage area only. Determination of the appropriate area to use for analysis (see Table 7) was based on a careful consideration of a number of relevant factors, including: 1) location of impaired waterbodies in the HUC-12 subwatershed; 2) land use type and distribution; 3) water quality monitoring data; and 4) the assessment status of other waterbodies in the HUC-12 subwatershed.

**Table 7 Determination of Analysis Areas for TMDL Development**

| HUC-12 Subwatershed (06010206____) | Impaired Waterbody | Area |
|---|---|---|
| 0205 | Little Mulberry Creek | HUC-12 |
| | Mulberry Creek | |
| 0301 | Little Creek | DA |
| 0302 | Russell Creek | HUC-12 |
| 0304 | Gap Creek | DA |
| 0307 | Cawood Creek | DA |
| | Davis Creek | DA |
| | Russell Branch | DA |

Note: HUC-12 = HUC-12 Subwatershed
DA = Waterbody Drainage Area

## 8.3 TMDL Analysis Methodology

TMDLs for the Powell River Watershed were developed using load duration curves for analysis of impaired HUC-12 subwatersheds or specific waterbody drainage areas. A load duration curve (LDC) is a cumulative frequency graph that illustrates existing water quality conditions (as represented by loads calculated from monitoring data), how these conditions compare to desired targets, and the portion of the waterbody flow zone represented by these existing loads. Load duration curves are considered to be well suited for analysis of periodic monitoring data collected by grab sample. LDCs were developed at monitoring site locations in impaired waterbodies and daily loading functions were expressed for TMDLs, WLAs, LAs, and MOS. In addition, load reductions (PLRGs) for each flow zone were calculated for prioritization of implementation measures according to the methods described in Appendix E.

## 8.4 Critical Conditions and Seasonal Variation

The critical condition for non-point source E. coli loading is an extended dry period followed by a rainfall runoff event. During the dry weather period, E. coli bacteria builds up on the land surface, and is washed off by rainfall. The critical condition for point source loading occurs during periods of low streamflow when dilution is minimized. Both conditions are represented in the TMDL analyses.

The ten-year period from October 1, 1997 to September 30, 2007 was used to simulate flow. This 10-year period contained a range of hydrologic conditions that included both low and high streamflows. Critical conditions are accounted for in the load duration curve analyses by using the entire period of flow and water quality data available for the impaired waterbodies.

In all subwatersheds, water quality data have been collected during most flow ranges. For each Subwatershed, the critical flow zone has been identified based on the incremental levels of impairment relative to the target loads. Based on the location of the water quality exceedances on the load duration curves and the distribution of critical flow zones, no one delivery mode for E. coli appears to be dominant for waterbodies in the Powell River watershed (see Section 9.1.2 and 9.1.3 and Table 8).

Seasonal variation was incorporated in the load duration curves by using the entire simulation period and all water quality data collected at the monitoring stations. The water quality data were collected during all seasons.

## 8.5 Margin of Safety

There are two methods for incorporating MOS in TMDL analysis: a) implicitly incorporate the MOS using conservative model assumptions; or b) explicitly specify a portion of the TMDL as the MOS and use the remainder for allocations. For development of pathogen TMDLs in the Powell River Watershed, an explicit MOS, equal to 10% of the E. coli water quality targets (ref.: Section 5.0), was utilized for determination of WLAs and LAs:

| | |
|---|---|
| Instantaneous Maximum (lakes, reservoirs, State Scenic Rivers, or Exceptional Tennessee Waters waterbodies): | MOS = 49 CFU/100 ml |
| Instantaneous Maximum (all other waterbodies): | MOS = 94 CFU/100 ml |
| 30-Day Geometric Mean: | MOS = 13 CFU/100 ml |

## 8.6 Determination of TMDLs

E. coli daily loading functions were calculated for impaired segments in the Powell River watershed using LDCs to evaluate compliance with the single maximum target concentrations according to the procedure in Appendix C. These TMDL loading functions for impaired segments and subwatersheds are shown in Table 8.

## 8.7 Determination of WLAs & LAs

WLAs for MS4s and LAs for precipitation induced sources of E. coli loading were determined according to the procedures in Appendix C. These allocations represent the available loading <u>after application of the explicit MOS</u>. WLAs for existing WWTFs are equal to their existing NPDES permit limits. Since WWTF permit limits require that E. coli concentrations must comply with water quality criteria (TMDL targets) at the point of discharge (with few exceptions in Tennessee) and recognition that loading from these facilities are generally small in comparison to other loading sources, further reductions were not considered to be warranted. WLAs for CAFOs and LAs for “other direct sources” (non-precipitation induced) are equal to zero. WLAs, & LAs are summarized in Table 8.

**Table 8 TMDLs, WLAs, & LAs expressed as daily loads for Impaired Waterbodies in the Powell River Watershed (HUC 06010206)**

| HUC-12 Subwatershed (06010206__) or Drainage Area (DA) | Impaired Waterbody Name | Impaired Waterbody ID | TMDL | MOS | WLAs | | | | LAs |
|---|---|---|---|---|---|---|---|---|---|
| | | | | | WWTFs [a] | Collection Systems | CAFOs | MS4s [b] | |
| | | | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/d/ac] | [CFU/d/ac] |
| 0205 | Little Mulberry Creek | TN06010104007 – 0710 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $7.877 \times 10^{6} \times Q$ |
| | Mulberry Creek | TN06010104007 – 0700 | $1.20 \times 10^{10} \times Q$ | $1.20 \times 10^{9} \times Q$ | NA | NA | NA | NA | $7.425 \times 10^{5} \times Q$ |
| 0301 (DA) | Little Creek | TN06010104007 – 0100 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $4.545 \times 10^{6} \times Q$ |
| 0302 | Russell Creek | TN06010104008 – 1000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | $2.315 \times 10^{10}$ | 0 | NA | NA | $1.913 \times 10^{6} \times Q - 2.140 \times 10^{6}$ |
| 0304 (DA) | Gap Creek | TN06010104006 – 0200 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | $1.843 \times 10^{9}$ | 0 | NA | NA | $2.170 \times 10^{6} \times Q - 1.933 \times 10^{5}$ |
| 0307 (DA) | Cawood Branch | TN06010104026 – 0100 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $9.668 \times 10^{6} \times Q$ |
| 0307 (DA) | Russell Branch | TN06010104026 – 0200 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $1.232 \times 10^{7} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 1000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $9.998 \times 10^{5} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 2000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $1.607 \times 10^{6} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 3000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $2.777 \times 10^{6} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 4000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $1.088 \times 10^{7} \times Q$ |

Notes: NA = Not Applicable.
Q = Mean Daily In-stream Flow (cfs).

a. WLAs for WWTFs are expressed as E. coli loads (CFU/day). All current and future WWTFs must meet water quality standards as specified in their NPDES permit.

b. Applies to any MS4 discharge loading in the subwatershed. Future MS4s will be assigned waste load allocations (WLAs) consistent with load allocations (LAs) assigned to precipitation induced nonpoint sources.

## 9.0 IMPLEMENTATION PLAN

The TMDLs, WLAs, and LAs developed in Section 8 are intended to be the first phase of a long-term effort to restore the water quality of impaired waterbodies in the Powell River watershed through reduction of excessive E. coli loading. Adaptive management methods, within the context of the State's rotating watershed management approach, will be used to modify TMDLs, WLAs, and LAs as required to meet water quality goals.

TMDL implementation activities will be accomplished within the framework of Tennessee's Watershed Approach (ref: http://www.state.tn.us/environment/wpc/watershed/ ). The Watershed Approach is based on a five-year cycle and encompasses planning, monitoring, assessment, TMDLs, WLAs/LAs, and permit issuance. It relies on participation at the federal, state, local and non-governmental levels to be successful.

### 9.1 Application of Load Duration Curves for Implementation Planning

The Load Duration Curve (LDC) methodology (Appendix C) is a form of water quality analysis and presentation of data that aids in guiding implementation by targeting management strategies for appropriate flow conditions. One of the strengths of this method is that it can be used to interpret possible delivery mechanisms of E. coli by differentiating between point and non-point source problems. The load duration curve analysis can be utilized for implementation planning. See Cleland (2003) for further information on duration curves and TMDL development, and: http://www.tmdls.net/tipstools/docs/TMDLsCleland.pdf .

#### 9.1.1 Flow Zone Analysis for Implementation Planning

A major advantage of the duration curve framework in TMDL development is the ability to provide meaningful connections between allocations and implementation efforts (USEPA, 2006). Because the flow duration interval serves as a general indicator of hydrologic condition (i.e., wet versus dry and to what degree), allocations and reduction goals can be linked to source areas, delivery mechanisms, and the appropriate set of management practices. The use of duration curve zones (e.g., high flow, moist, mid-range, dry, and low flow) allows the development of allocation tables (USEPA, 2006) (Appendix E), which can be used to guide potential implementation actions to most effectively address water quality concerns.

For the purposes of implementation strategy development, available E. coli data are grouped according to flow zones, with the number of flow zones determined by the HUC-12 subwatershed or drainage area size, the total contributing area (for non-headwater HUC-12s), and/or the baseflow characteristics of the waterbody. In general, for drainage areas greater than 40 square miles, the duration curves will be divided into five zones (Figure 11): high flows (exceeded 0-10% of the time), moist conditions (10-40%), median or mid-range flows (40-60%), dry conditions (60-90%), and low flows (90-100%). For smaller drainage areas, flows occurring in the low flow zone (baseflow conditions) are often extremely low and difficult to measure accurately. In many small drainage areas, extreme dry conditions are characterized by zero flow for a significant percentage of time. For this reason, the low flow zone is best characterized as a broader range of conditions (or percent time) with subsequently fewer flow zones. Therefore, for most HUC-12 subwatershed drainage areas less than 40 square miles, the duration curves will be divided into four zones: high flows (exceeded 0-10% of the time), moist conditions (10-40%), median or mid-range flows (40-70%), and

low flows (70-100%). Some small (<40 $mi^2$) waterbody drainage areas have sustained baseflow (no zero flows) throughout their period of record. For these waterbodies, the duration curves will be divided into five zones.

Given adequate data, results (allocations and percent load reduction goals) will be calculated for all flow zones; however, less emphasis is placed on the upper 10% flow range for pathogen (E. coli) TMDLs and implementation plans. The highest 10 percent flows, representing flood conditions, are considered non-recreational conditions: unsafe for wading and swimming. Humans are not expected to enter the water due to the inherent hazard from high depths and velocities during these flow conditions. As a rule of thumb, the *USGS Field Manual for the Collection of Water Quality Data* (Lane, 1997) advises its personnel not to attempt to wade a stream for which values of depth (ft) multiplied by velocity (ft/s) equal or exceed 10 $ft^2/s$ to collect a water sample. Few observations are typically available to estimate loads under these adverse conditions due to the difficulty and danger of sample collection. Therefore, in general, the 0-10% flow range is beyond the scope of pathogen TMDLs and subsequent implementation strategies.

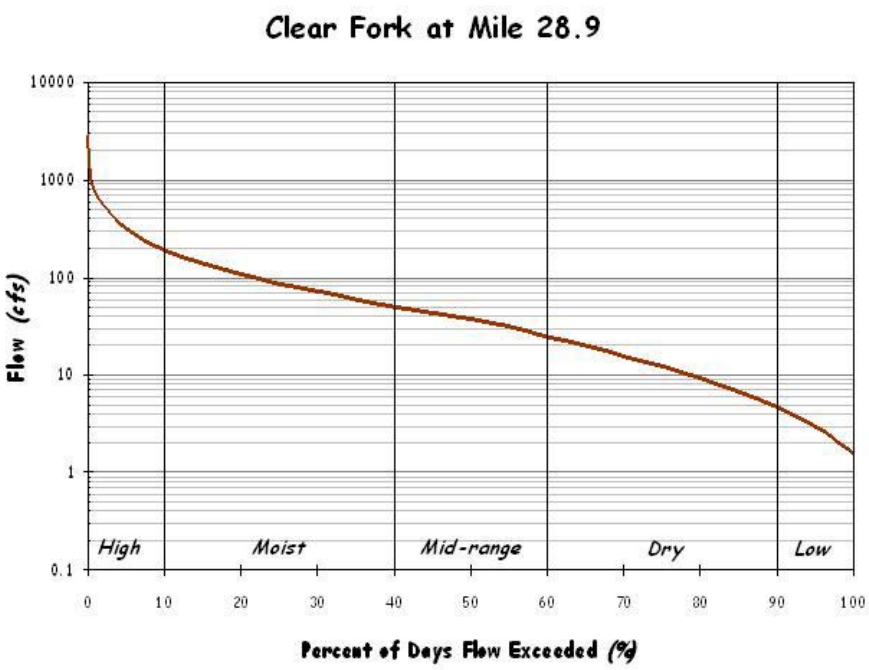


**Figure 11. Five-Zone Flow Duration Curve for Clear Fork at RM 28.9**

### 9.1.2 Existing Loads and Percent Load Reductions

Each impaired waterbody has a characteristic set of pollutant sources and existing loading conditions that vary according to flow conditions. In addition, maximum allowable loading (assimilative capacity) of a waterbody varies with flow. Therefore, existing loading, allowable loading, and percent load reduction expressed at a single location on the LDC (for a single flow condition) do not appropriately represent the TMDL in order to address all sources under all flow conditions (i.e., at all times) to satisfy implementation objectives. The LDC approach provides a methodology for determination of assimilative capacity and existing loading conditions of a waterbody for each flow zone. Subsequently, each flow zone, and the sources contributing to impairment under the corresponding flow conditions, can be evaluated independently. Lastly, the critical flow zone (with the highest percent load reduction goal) can be identified for prioritization of implementation actions.

Existing loading is calculated for each individual water quality sample as the product of the sample flow (cfs) times the single sample E. coli concentration (times a conversion factor). A percent load reduction is calculated for each water quality sample as that required to reduce the existing loading to the product of the sample flow (cfs) times the single sample maximum water quality standard (times a conversion factor). For samples with negative percent load reductions (non-exceedance: concentration below the single sample maximum water quality criterion), the percent reduction is assumed to be zero. The percent load reduction goal (PLRG) for a given flow zone is calculated a s the mean of all the percent load reductions for a given flow zone. See Appendix E.

### 9.1.3 Critical Conditions

The critical condition for each impaired waterbody is defined as the flow zone with the largest PLRG, excluding the “high flow” zone because these extremely high flows are not representative of recreational flow conditions, as described in Section 9.1.1. If the PLRG in this zone is greater than all the other zones, the zone with the second highest PLRG will be considered the critical flow zone. The critical conditions are such that if water quality standards were met under those conditions, they would likely be met overall.

## 9.2 Point Sources

### 9.2.1 NPDES Regulated Municipal and Industrial Wastewater Treatment Facilities

All present and future discharges from industrial and municipal wastewater treatment facilities are required to be in compliance with the conditions of their NPDES permits at all times, including elimination of bypasses and overflows. With few exceptions, in Tennessee, permit limits for treated sanitary wastewater require compliance with coliform water quality standards (ref: Section 5.0) prior to discharge. No additional reduction is required. WLAs for WWTFs are derived from facility design flows and permitted E. coli limits and are expressed as average loads in CFU per day.

### 9.2.2 NPDES Regulated Municipal Separate Storm Sewer Systems (MS4s)

For present and future regulated discharges from municipal separate storm sewer systems (MS4s), WLAs are and will be implemented through Phase I & II MS4 permits. These permits will require the development and implementation of a Storm Water Management Program (SWMP) that will reduce the discharge of pollutants to the "maximum extent practicable" and not cause or contribute

to violations of State water quality standards. Both the *NPDES General Permit for Discharges from Small Municipal Separate Storm Sewer Systems* (TDEC, 2003) and the TDOT individual MS4 permit (TNS077585) require SWMPs to include minimum control measures. The permits also contain requirements regarding control of discharges of pollutants of concern into impaired waterbodies, implementation of provisions of approved TMDLs, and descriptions of methods to evaluate whether storm water controls are adequate to meet the requirements of approved TMDLs.

For guidance on the six minimum control measures for MS4s regulated under Phase I or Phase II, a series of fact sheets are available at: http://cfpub1.epa.gov/npdes/stormwater/swfinal.cfm?program_id=6 .

For further information on Tennessee's *NPDES General Permit for Discharges from Small Municipal Separate Storm Sewer Systems*, see: http://state.tn.us/environment/wpc/ppo/TN%20Small%20MS4%20Modified%General%20Permit%202003.pdf .

In order to evaluate SWMP effectiveness and demonstrate compliance with specified WLAs, MS4s must develop and implement appropriate monitoring programs. An effective monitoring program could include:

- Effluent monitoring at selected outfalls that are representative of particular land uses or geographical areas that contribute to pollutant loading before and after implementation of pollutant control measures.
- Analytical monitoring of pollutants of concern (e.g., monthly) in receiving waterbodies, both upstream and downstream of MS4 discharges, over an extended period of time. In addition, intensive collection of pollutant monitoring data during the recreation season (June – September) at sufficient frequency to support calculation of the geometric mean.

When applicable, the appropriate Division of Water Pollution Control Environmental Field Office should be consulted for assistance in the determination of monitoring strategies, locations, frequency, and methods within 12 months after the approval date of TMDLs or designation as a regulated MS4. Details of the monitoring plans and monitoring data should be included in annual reports required by MS4 permits.

#### 9.2.3 NPDES Regulated Concentrated Animal Feeding Operations (CAFOs)

WLAs provided to most CAFOs will be implemented through NPDES Permit No. TNA000000, General NPDES Permit for *Class II Concentrated Animal Feeding Operation* or the facility's individual permit. Provisions of the general permit include development and implementation of Nutrient Management Plan (NMPs), requirements regarding land application BMPs, and requirements for CAFO liquid waste management systems. For further information, see: http://state.tn.us/environment/wpc/permits/cafo.shtml.

## 9.3 Nonpoint Sources

The Tennessee Department of Environment & Conservation has no direct regulatory authority over most nonpoint source (NPS) discharges. Reductions of E. coli loading from nonpoint sources will be achieved using a phased approach. Voluntary, incentive-based mechanisms will be used to implement NPS management measures in order to assure that measurable reductions in pollutant loadings can be achieved for the targeted impaired waters. Cooperation and active participation by the general public and various industry, business, and environmental groups is critical to successful implementation of TMDLs. There are links to a number of publications and information resources on EPA's Nonpoint Source Pollution web page (http://www.epa.gov/owow/nps/pubs.html ) relating to the implementation and evaluation of nonpoint source pollution control measures.

Local citizen-led and implemented management measures have the potential to provide the most efficient and comprehensive avenue for reduction of loading rates from nonpoint sources. An excellent example of stakeholder involvement is the Cumberland River Compact. The Cumberland River Compact is a non-profit group made up of businesses, individuals, community organizations, and agencies working in the Cumberland River watershed. Members of the Compact work with educators, landowners, contractors, marinas and other interested groups to coordinate informational education programs that encourage all of us to be better stewards of our water resources. The Compact works with local, state and federal agencies and officials to promote and strengthen cooperative working relationships and encourage the development of reliable, easy-to-understand data about water quality. Members of the Compact work with local communities to develop watershed forums where citizens come together to learn more about their watershed and participate in developing a shared vision for the future. The Compact also serves as a clearing-house of available public education programs to landowner assistance. Information regarding the accomplishments of the Cumberland River Compact is available at their website:

http://www.cumberlandrivercompact.org/.

### 9.3.1 Urban Nonpoint Sources

Management measures to reduce pathogen loading from urban nonpoint sources are similar to those recommended for MS4s (Sect. 9.2.2). Specific categories of urban nonpoint sources include stormwater, illicit discharges, septic systems, pet waste, and wildlife:

Stormwater: Most mitigation measures for stormwater are not designed specifically to reduce bacteria concentrations (ENSR, 2005). Instead, BMPs are typically designed to remove sediment and other pollutants. Bacteria in stormwater runoff are, however, often attached to particulate matter. Therefore, treatment systems that remove sediment may also provide reductions in bacteria concentrations.

Illicit discharges: Removal of illicit discharges to storm sewer systems, particularly of sanitary wastes, is an effective means of reducing pathogen loading to receiving waters (ENSR, 2005). These include intentional illegal connections from commercial or residential buildings, failing septic systems, and improper disposal of sewage from campers and boats.

Septic systems: When properly installed, operated, and maintained, septic systems effectively reduce pathogen concentrations in sewage. To reduce the release of pathogens, practices can be employed to maximize the life of existing systems, identify failed systems, and replace or remove failed systems (USEPA, 2005a). Alternatively, the installation of public sewers may be appropriate.

Pet waste: If the waste is not properly disposed of, these bacteria can wash into storm drains or directly into water bodies and contribute to pathogen impairment. Encouraging pet owners to properly collect and dispose of pet waste is the primary means for reducing the impact of pet waste (USEPA, 2002b).

Wildlife: Reducing the impact of wildlife on pathogen concentrations in waterbodies generally requires either reducing the concentration of wildlife in an area or reducing their proximity to the waterbody (ENSR, 2005). The primary means for doing this is to eliminate human inducements for congregation. In addition, in some instances population control measures may be appropriate.

Two additional urban nonpoint source resource documents provided by EPA are:

*National Management Measures to Control Nonpoint Source Pollution from Urban Areas* (http://www.epa.gov/owow/nps/urbanmm/index.html ) helps citizens and municipalities in urban areas protect bodies of water from polluted runoff that can result from everyday activities. The scientifically sound techniques techniques it presents are among the best practices known today. The guidance will also help states to implement their nonpoint source control programs and municipalities to implement their Phase II Storm Water Permit Programs (Publication Number EPA 841-B-05-004, November 2005).

*The Use of Best Management Practices (BMPs) in Urban Watersheds* (http://www.epa.gov/nrmrl/pubs/600r04184/600r04184chap1.pdf ) is a comprehensive literature review on commonly used urban watershed Best Management Practices (BMPs) that heretofore was not consolidated. The purpose of this document is to serve as an information source to individuals and agencies/municipalities/watershed management groups/etc. on the existing state of BMPs in urban stormwater management (Publication Number EPA/600/R-04/184, September 2004).

### 9.3.2 Agricultural Nonpoint Sources

BMPs have been utilized in the Powell River watershed to reduce the amount of coliform bacteria transported to surface waters from agricultural sources. These BMPs (e.g., animal waste management systems, waste utilization, stream stabilization, fencing, heavy use area treatment, livestock exclusion, etc.) may have contributed to reductions in in-stream concentrations of coliform bacteria in one or more Powell River watershed E. coli-impaired subwatersheds during the TMDL evaluation period. The Natural Resources Conservation Service (NRCS) keeps a database of BMPs implemented in Tennessee. Those listed in the Powell River watershed are shown in Figure 12. It is recommended that additional information (e.g., livestock access to streams, manure application practices, etc.) be provided and evaluated to better identify and quantify agricultural sources of coliform bacteria loading in order to minimize uncertainty in future modeling efforts.

It is further recommended that additional BMPs be implemented and monitored to document performance in reducing coliform bacteria loading to surface waters from agricultural sources. Demonstration sites for various types of BMPs should be established and maintained, and their performance (in source reduction) evaluated over a period of at least two years prior to recommendations for utilization for subsequent implementation. E. coli sampling and monitoring are recommended during low-flow (baseflow) and storm periods at sites with and without BMPs and/or before and after implementation of BMPs.

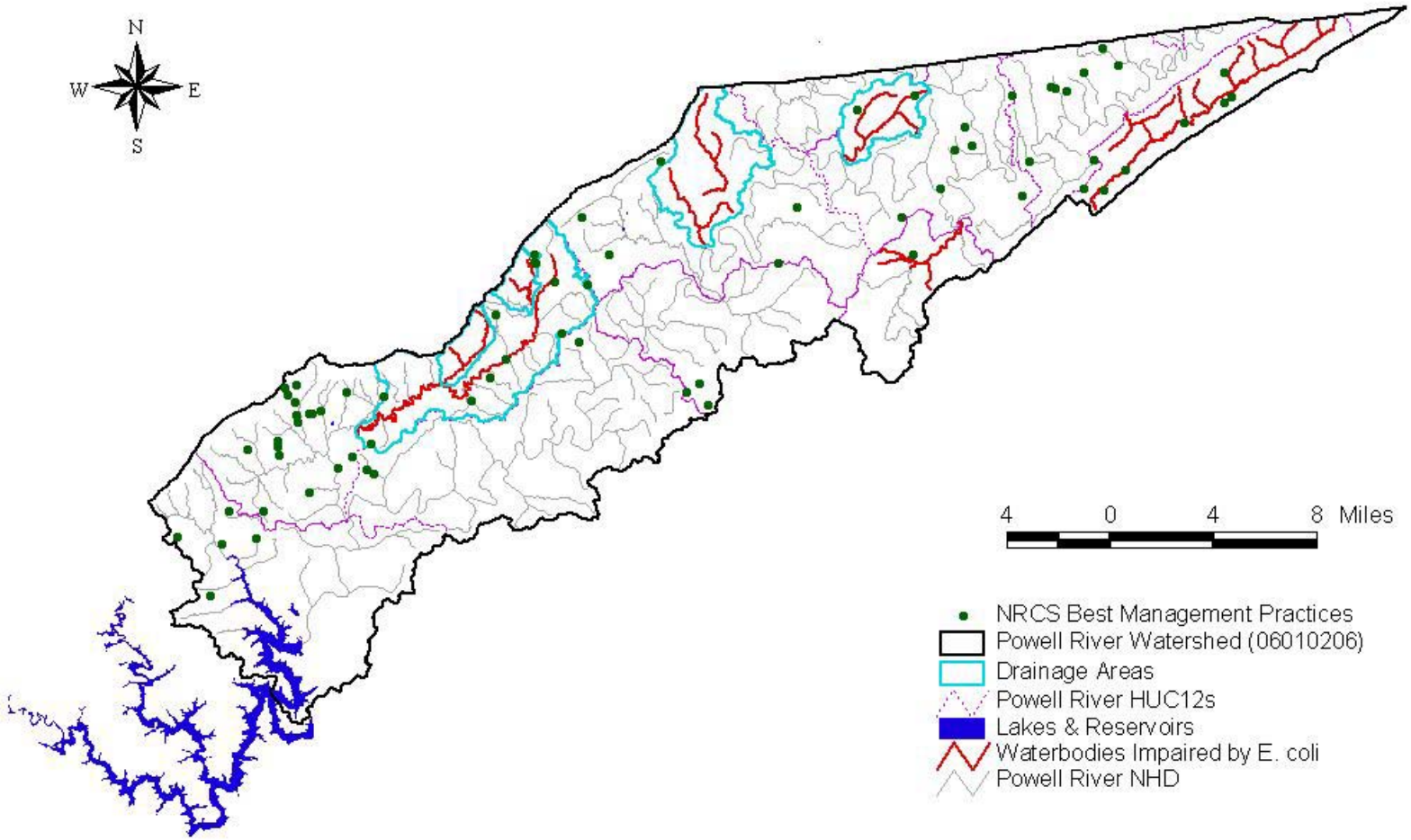


**Figure 12. NRCS Best Management Practices located in the Powell River Watershed.**

For additional information on agricultural BMPs in Tennessee, see: http://state.tn.us/agriculture/nps/bmpa.ntml .

An additional agricultural nonpoint source resource provided by EPA is *National Management Measures to Control Nonpoint Source Pollution from Agriculture* (http://www.epa.gov/owow/nps/agmm/index.html ): a technical guidance and reference document for use by State, local, and tribal managers in the implementation of nonpoint source pollution management programs. It contains information on the best available, economically achievable means of reducing pollution of surface and groundwater from agriculture (EPA 841-B-03-004, July 2003).

### 9.3.3 Other Nonpoint Sources

Additional nonpoint source references (not specifically addressing urban and/or agricultural sources) provided by EPA include:

*National Management Measures to Control Nonpoint Source Pollution from Forestry* (http://www.epa.gov/owow/nps/forestrymgmt/ ) helps forest owners protect lakes and streams from polluted runoff that can result from forestry activities. These scientifically sound techniques are the best practices known today. The report will also help states to implement their nonpoint source control programs (EPA 841-B-05-001, May 2005).

In addition, the EPA website, http://www.epa.gov/owow/nps/bestnpsdocs.html , contains a list of guidance documents endorsed by the Nonpoint Source Control Branch at EPA headquarters. The list includes documents addressing urban, agriculture, forestry, marinas, stream restoration, nonpoint source monitoring, and funding.

## 9.4 Additional Monitoring

Additional monitoring and assessment activities are recommended to determine whether implementation of TMDLs, WLAs, & LAs in tributaries and upstream reaches will result in achievement of in-stream water quality targets for E. coli.

### 9.4.1 Water Quality Monitoring

Activities recommended for the Powell River watershed:

> Verify the assessment status of stream reaches identified on the Final 2008 303(d) List as impaired due to E. coli. If it is determined that these stream reaches are still not fully supporting designated uses, then sufficient data to enable development of TMDLs should be acquired. TMDLs will be revisited on 5-year watershed cycle as described above.
>
> Evaluate the effectiveness of implementation measures (see Sect. 9.6). Includes BMP performance analysis and monitoring by permittees and stakeholders. Where required TMDL loading reduction has been fully achieved, adequate data to support delisting should be collected.

Provide additional data to clarify status of ambiguous sites (e.g., geometric mean data) for potential listing. Analyses of existing data at several monitoring sites on unlisted waterbodies in the Powell River watershed suggest levels of impairment. Therefore, additional data are required for listing determination.

Provide additional data to clarify status of ambiguous sites (e.g. geometric mean data). Analysis of existing data for Little Creek and Gap Creek suggests that there is reasonable potential for impairment. However, insufficient data was available to calculate a load reduction using traditional methods. Additional data will be required to either confirm impairment or recommend de-listing.

Continue ambient (long-term) monitoring at appropriate sites and key locations.

Comprehensive water quality monitoring activities include sampling during all seasons and a broad range of flow and meteorological conditions. In addition, collection of E. coli data at sufficient frequency to support calculation of the geometric mean, as described in Tennessee's General Water Quality Criteria (TDEC, 2004a), is encouraged. Finally, for individual monitoring locations, where historical E. coli data are greater than 1000 colonies/100 mL (or future samples are anticipated to be), a 1:100 dilution should be performed as described in Protocol A of the *Quality System Standard Operating Procedure for Chemical and Bacteriological Sampling of Surface Water* (TDEC, 2004).

### 9.4.2 Source Identification

An important aspect of E. coli load reduction activities is the accurate identification of the actual sources of pollution. In cases where the sources of E. coli impairment are not readily apparent, Microbial Source Tracking (MST) is one approach to determining the sources of fecal pollution and E. coli affecting a waterbody. Those methods that use bacteria as target organisms are also known as Bacterial Source Tracking (BST) methods. This technology is recommended for source identification in E. coli impaired waterbodies.

Bacterial Source Tracking is a collective term used for various emerging biochemical, chemical, and molecular methods that have been developed to distinguish sources of human and non-human fecal pollution in environmental samples (Shah, 2004). In general, these methods rely on genotypic (also known as "genetic fingerprinting"), or phenotypic (relating to the physical characteristics of an organism) distinctions between the bacteria of different sources. Three primary genotypic techniques are available for BST: ribotyping, pulsed field gel electrophoresis (PFGE), and polymerase chain reaction (PCR). Phenotypic techniques generally involve an antibiotic resistance analysis (Hyer, 2004).

The USEPA has published a fact sheet that discusses BST methods and presents examples of BST application to TMDL development and implementation (USEPA, 2002b). Various BST projects and descriptions of the application of BST techniques used to guide implementation of effective BMPs to remove or reduce fecal contamination are presented. The fact sheet can be found on the following EPA website: http://www.epa.gov/owm/mtb/bacsortk.pdf.

A multi-disciplinary group of researchers at the University of Tennessee, Knoxville (UTK) has developed and tested a series of different microbial assay methods based on real-time PCR to detect fecal bacterial concentrations and host sources in water samples (Layton, 2006). The

assays have been used in a study of fecal contamination and have proven useful in identification of areas where cattle represent a significant fecal input and in development of BMPs. It is expected that these types of assays could have broad applications in monitoring fecal impacts from Animal Feeding Operations, as well as from wildlife and human sources. Additional information can be found on the following UTK website: http://web.utk.edu/~hydro/JournalPapers/Layton06AEM.pdf .

BST technology was utilized in a study conducted in Stock Creek (Little River watershed) (Layton, 2004). Microbial source tracking using real-time PCR assays to quantify *Bacteroides* 16S rRNA genes was used to determine the percent of fecal contamination attributable to cattle. E. coli loads attributable to cattle were calculated for each of nine sampling sites in the Stock Creek subwatershed on twelve sampling dates. At the site on High Bluff Branch (tributary to Stock Creek), none of the sample dates had E. coli loads attributable to cattle above the threshold. This suggests that at this site removal of E. coli attributable to cattle would have little impact on the total E. coli loads. The E. coli load attributable to cattle made a large contribution to the total E. coli load at each of the eight remaining sampling sites. At two of the sites (STOCK005.3KN and GHOLL000.6KN), 50–75% of the E. coli attributable to cattle loads alone was above the 126 CFU/100mL threshhold. This suggests that removal of the E. coli attributable to cattle at these sites would reduce the total E. coli load to acceptable limits.

## 9.5 Source Area Implementation Strategy

Implementation strategies are organized according to the dominant landuse type and the sources associated with each (Table 9 and Appendix E). Each HUC-12 subwatershed is grouped and targeted for implementation based on this source area organization. Three primary categories are identified: predominantly urban, predominantly agricultural, and mixed urban/agricultural. See Appendix A for information regarding landuse distributation of impaired subwatersheds. For the purpose of implementation evaluation, urban is defined as residential, commercial, and industrial landuse areas with predominant source categories such as point sources (WWTFs), collection systems/septic systems (including SSOs and CSOs), and urban stormwater runoff associated with MS4s. Agricultural is defined as cropland and pasture, with predominant source categories associated with livestock and manure management activities. A fourth category (infrequent) is associated with forested (including non-agricultural undeveloped and unaltered [by humans]) landuse areas with the predominant source category being wildlife.

All impaired waterbodies and corresponding HUC-12 subwatersheds or drainage areas have been classified according to their respective source area types in Table 9. The implementation for each area will be prioritized according to the guidance provided in Sections 9.5.1 and 9.5.2, below. For all impaired waterbodies, the determination of source area types serves to identify the predominant sources contributing to impairment (i.e., those that should be targeted initially for implementation). However, it is not intended to imply that sources in other landuse areas are not contributors to impairment and/or to grant an exemption from addressing other source area contributions with implementation strategies and corresponding load reduction. For mixed use areas, implementation will follow the guidance established for both urban and agricultural areas, at a minimum.

Appendix E provides source area implementation examples for urban and agricultural subwatersheds, development of percent load reduction goals, and determination of critical flow zones (for implementation prioritization) for E. coli impaired waterbodies. Load duration curve analyses (TMDLs, WLAs, LAs, and MOS) and percent load reduction goals for all flow zones for all E. coli impaired waterbodies in the Powell River watershed are summarized in Table E-19.

**Table 9. Source area types for waterbody drainage area analyses.**

| Waterbody NameD | Source Area Type* | | | |
|---|---|---|---|---|
| | Urban | Agricultural | Mixed | Forested |
| Little Mulberry Creek | | Ò | | |
| Mulberry Creek | | Ò | | |
| Little Creek | | | Ò | |
| Russell Creek | | | Ò | |
| Gap Creek | | | Ò | |
| Cawood Branch | | Ò | | |
| Davis Creek | | Ò | | |
| Russell Branch | | Ò | | |

* All waterbodies potentially have significant source contributions from other source type/landuse areas.

### 9.5.1 Urban Source Areas

For impaired waterbodies and corresponding HUC-12 subwatersheds or drainage areas classified as predominantly urban, implementation strategies for E. coli load reduction will initially and primarily target source categories similar to those listed in Table 10 (USEPA, 2006). Table 10 presents example urban area management practices and the corresponding potential relative effectiveness under each of the hydrologic flow zones. Each implementation strategy addresses a range of flow conditions and targets point sources, non-point sources, or a combination of each. For each waterbody, the existing loads and corresponding PLRG for each flow zone are calculated according to the method described in Section E.4. The resulting determination of the critical flow zone further focuses the types of urban management practices appropriate for development of an effective load reduction strategy for a particular waterbody.

**Table 10. Example Urban Area Management Practice/Hydrologic Flow Zone Considerations.**

| Management Practice | Duration Curve Zone (Flow Zone) | | | | |
|---|---|---|---|---|---|
| | **High** | **Moist** | **Mid-Range** | **Dry** | **Low** |
| **Bacteria source reduction** | | | | | |
| Remove illicit discharges | | | L | M | H |
| Address pet & wildlife waste | | H | M | M | L |
| **Combined sewer overflow management** | | | | | |
| Combined sewer separation | | H | M | L | |
| CSO prevention practices | | H | M | L | |
| **Sanitary sewer system** | | | | | |
| Infiltration/Inflow mitigation | H | M | L | L | |
| Inspection, maintenance, and repair | | L | M | H | H |
| SSO repair/abatement | H | M | L | | |
| Illegal cross-connections | | | | | |
| **Septic system management** | | | | | |
| Managing private systems | | L | M | H | M |
| Replacing failed systems | | L | M | H | M |
| Installing public sewers | | L | M | H | M |
| **Storm water infiltration/retention** | | | | | |
| Infiltration basin | | L | M | H | |
| Infiltration trench | | L | M | H | |
| Infiltration/Biofilter swale | | L | M | H | |
| **Storm Water detention** | | | | | |
| Created wetland | | H | M | L | |

**Table 10 (cont'd). Example Urban Area Management Practice/Hydrologic Flow Zone Considerations.**

| Management Practice | Duration Curve Zone (Flow Zone) | | | | |
|---|---|---|---|---|---|
| | High | Moist | Mid-Range | Dry | Low |
| **Low impact development** | | | | | |
| Disconnecting impervious areas | | L | M | H | |
| Bioretention | L | M | H | H | |
| Pervious pavement | | L | M | H | |
| Green Roof | | L | M | H | |
| Buffers | | H | H | H | |
| **New/existing on-site wastewater treatment systems** | | | | | |
| Permitting & installation programs | | L | M | H | M |
| Operation & maintenance programs | | L | M | H | M |
| **Other** | | | | | |
| Point source controls | | L | M | H | H |
| Landfill control | | L | M | H | |
| Riparian buffers | | H | H | H | |
| Pet waste education & ordinances | | M | H | H | L |
| Wildlife management | | M | H | H | L |
| Inspection & maintenance of BMPs | L | M | H | H | L |
| **Note**: Potential relative importance of management practice effectiveness under given hydrologic condition (*H: High, M: Medium, L: Low*) | | | | | |

### 9.5.2 Agricultural Source Areas

For impaired waterbodies and corresponding HUC-12 subwatersheds or drainage areas classified as predominantly agricultural, implementation strategies for E. coli load reduction will initially and primarily target source categories similar to those listed in Table 11 (USDA, 1988). Table 11 present example agricultural area management practices and the corresponding potential relative effectiveness under each of the hydrologic flow zones. Each implementation strategy addresses a range of flow conditions and targets point sources, non-point sources, or a combination of each. For each waterbody, the existing loads and corresponding PLRG for each flow zone are calculated according to the method described in Section E.4. The resulting determination of the critical flow zone further focuses the types of agricultural management practices appropriate for development of an effective load reduction strategy for a particular waterbody.

### 9.5.3 Forestry Source Areas

There are no impaired waterbodies with corresponding HUC-12 subwatersheds or drainage areas classified as source area type predominantly forested, with the predominant source category being wildlife, in the Powell River watershed.

**Table 11. Example Agricultural Area Management Practice/Hydrologic Flow Zone Considerations.**

| Flow Condition | High | Moist | Mid-range | Dry | Low |
|---|---|---|---|---|---|
| % Time Flow Exceeded | 0-10 | 10-40 | 40-60 | 60-90 | 90-100 |
| **Grazing Management** | | | | | |
| Prescribed Grazing (528A) | **H** | **H** | **M** | **L** | |
| Pasture & Hayland Mgmt (510) | **H** | **H** | **M** | **L** | |
| Deferred Grazing (352) | **H** | **H** | **M** | **L** | |
| Planned Grazing System (556) | **H** | **H** | **M** | **L** | |
| Proper Grazing Use (528) | **H** | **H** | **M** | **L** | |
| Proper Woodland Grazing (530) | **H** | **H** | **M** | **L** | |
| **Livestock Access Limitation** | | | | | |
| Livestock Exclusion (472) | | | **M** | **H** | **H** |
| Fencing (382) | | | **M** | **H** | **H** |
| Stream Crossing | | | **M** | **H** | **H** |
| **Alternate Water Supply** | | | | | |
| Pipeline (516) | | | **M** | **H** | **H** |
| Pond (378) | | | **M** | **H** | **H** |
| Trough or Tank (614) | | | **M** | **H** | **H** |
| Well (642) | | | **M** | **H** | **H** |
| Spring Development (574) | | | **M** | **H** | **H** |
| **Manure Management** | | | | | |
| Managing Barnyards | **H** | **H** | **M** | **L** | |
| Manure Transfer (634) | **H** | **H** | **M** | **L** | |
| Land Application of Manure | **H** | **H** | **M** | **L** | |
| Composting Facility (317) | **H** | **H** | **M** | **L** | |
| **Vegetative Stabilization** | | | | | |
| Pasture & Hayland Planting (512) | **H** | **H** | **M** | **L** | |
| Range Seeding (550) | **H** | **H** | **M** | **L** | |
| Channel Vegetation (322) | **H** | **H** | **M** | **L** | |
| Brush (& Weed) Mgmt (314) | **H** | **H** | **M** | **L** | |

**Table 11 (cont'd). Example Agricultural Area Management Practice/Hydrologic Flow Zone Considerations.**

| Flow Condition | High | Moist | Mid-range | Dry | Low |
|---|---|---|---|---|---|
| % Time Flow Exceeded | 0-10 | 10-40 | 40-60 | 60-90 | 90-100 |
| **Vegetative Stabilization (cont'd)** | | | | | |
| Conservation Cover (327) | | **H** | **H** | **H** | |
| Riparian Buffers (391) | | **H** | **H** | **H** | |
| Critical Area Planting (342) | | **H** | **H** | **H** | |
| Wetland restoration (657) | | **H** | **H** | **H** | |
| **CAFO Management** | | | | | |
| Waste Management System (312) | **H** | **H** | **M** | | |
| Waste Storage Structure (313) | **H** | **H** | **M** | | |
| Waste Storage Pond (425) | **H** | **H** | **M** | | |
| Waste Treatment Lagoon (359) | **H** | **H** | **M** | | |
| Mulching (484) | **H** | **H** | **M** | | |
| Waste Utilization (633) | **H** | **H** | **M** | | |
| Water & Sediment Control Basin (638) | **H** | **H** | **M** | | |
| Filter Strip (393) | **H** | **H** | **M** | | |
| Sediment Basin (350) | **H** | **H** | **M** | | |
| Grassed Waterway (412) | **H** | **H** | **M** | | |
| Diversion (362) | **H** | **H** | **M** | | |
| Heavy Use Area Protection (561) | | | | | |
| Constructed Wetland (656) | | | | | |
| Dikes (356) | **H** | **H** | **M** | | |
| Lined Waterway or Outlet (468) | **H** | **H** | **M** | | |
| Roof Runoff Mgmt (558) | **H** | **H** | **M** | | |
| Floodwater Diversion (400) | **H** | **H** | **M** | | |
| Terrace (600) | **H** | **H** | **M** | | |
| Potential for source area contribution under given hydrologic condition (H: High; M: Medium; L: Low) | | | | | |

Note: Numbers in parentheses are the U.S. Soil Conservation Service practice number.

## 9.6 Evaluation of TMDL Implementation Effectiveness

Evaluation of the effectiveness of TMDL implementation strategies should be conducted on multiple levels, as appropriate:

- HUC-12 or waterbody drainage area (i.e., TMDL analysis location)
- Subwatersheds or intermediate sampling locations
- Specific landuse areas (urban, pasture, etc.)
- Specific facilities (WWTF, CAFO, uniquely identified portion of MS4, etc.)
- Individual BMPs

In order to conduct an implementation effectiveness analysis on measures to reduce E. coli source loading, monitoring results should be evaluated in one of several ways. Sampling results can be compared to water quality standards (e.g., load duration curve analysis) for determination of impairment status, results can be compared on a before and after basis (temporal), or results can be evaluated both upstream and downstream of source reduction measures or source input (spatial). Considerations include period of record, data collection frequency, representativeness of data, and sampling locations.

In general, periods of record greater than 5 years (given adequate sampling frequency) can be evaluated for determination of relative change (trend analysis). For watershed in second or successive TMDL cycles, data collected from multiple cycles can be compared. If implementation efforts have been initiated to reduce loading, evaluation of routine monitoring data may indicate improving or worsening conditions over time and corresponding effectiveness of implementation efforts.

Water quality data for implementation effectiveness analysis can be presented in multiple ways. For example, Figure 13 shows fecal coliform concentration data statistics for Oostanaula Creek at mile 28.4 (Hiwassee River watershed) for a historical (2002) TMDL analysis period versus a recent post-implementation period of sampling data (revised TMDL). The individual flow zone analyses are presented in a box and whisker plot of recent [2] versus historical [1] data. Figure 14 shows a load duration curve analysis (of recent versus historical data) of fecal coliform loading statistics for Oostanaula Creek. Lastly, Figure 15 shows best fit curve analyses of flow (percent time exceeded) versus fecal coliform loading relationships (regressions) plotted against the LDC of the single sample maximum water quality standard. Note that Figures 13-15 present the same data, from approved TMDLs (2 cycles), each clearly illustrating improving conditions between historical and recent periods.

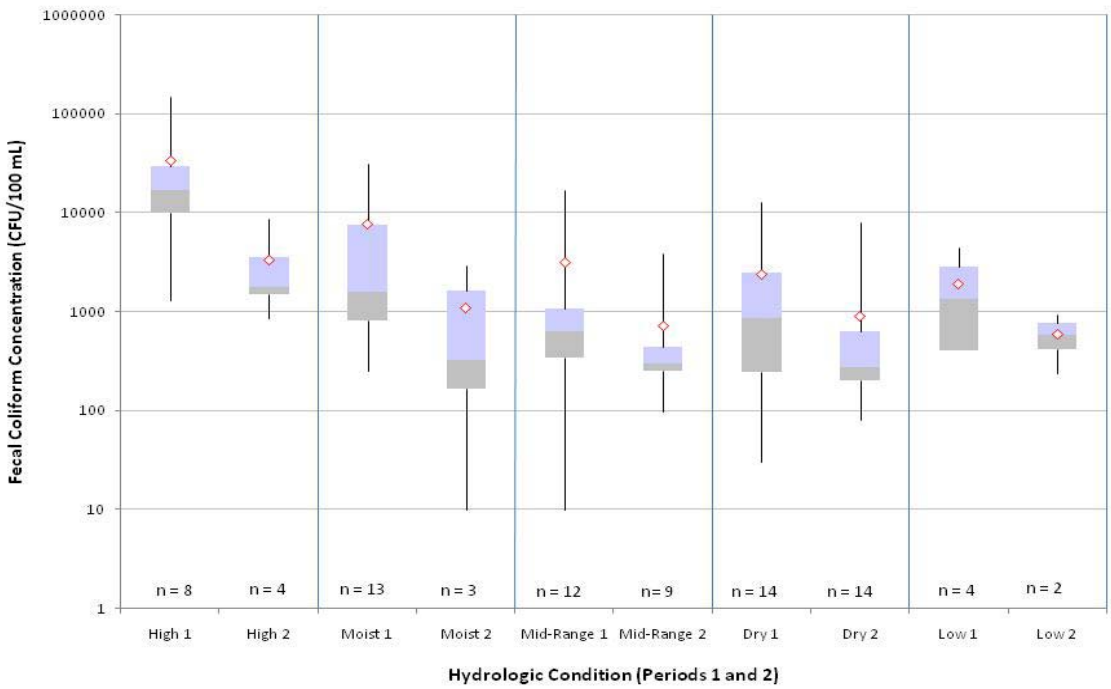


**Figure 13. Oostanaula Creek TMDL implementation effectiveness (box and whisker plot).**

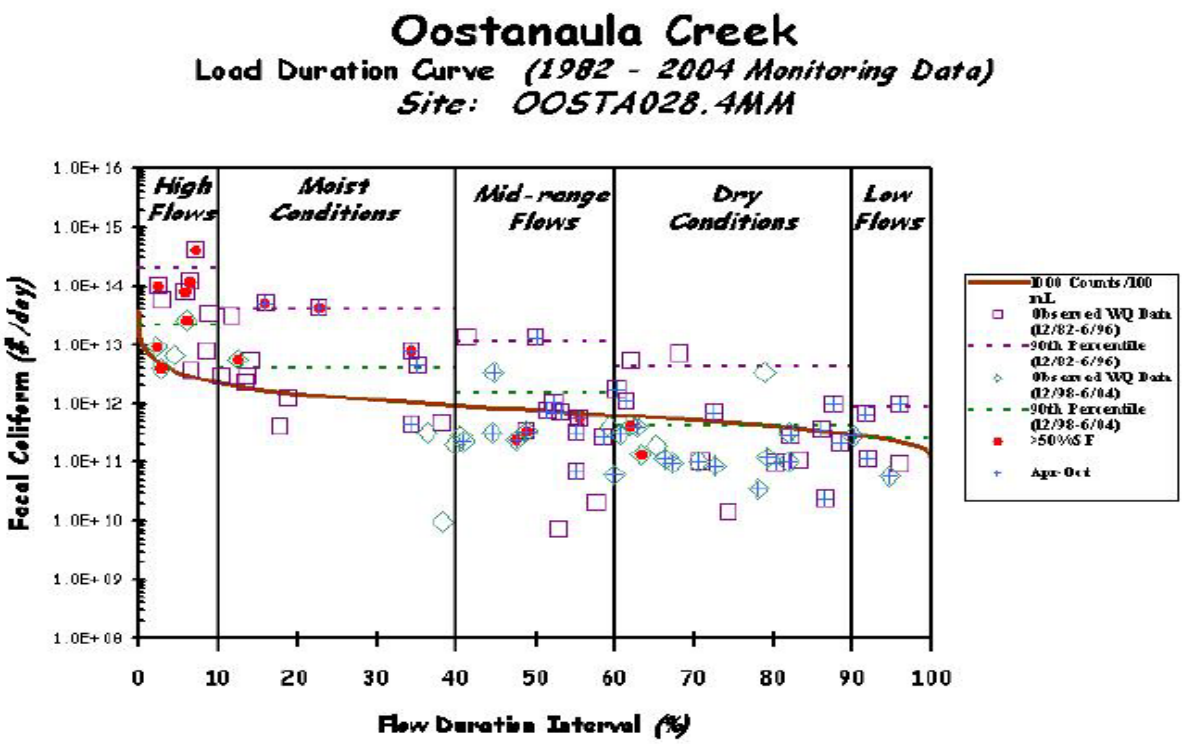


**Figure 14. Oostanaula Creek TMDL implementation effectiveness (LDC analysis).**

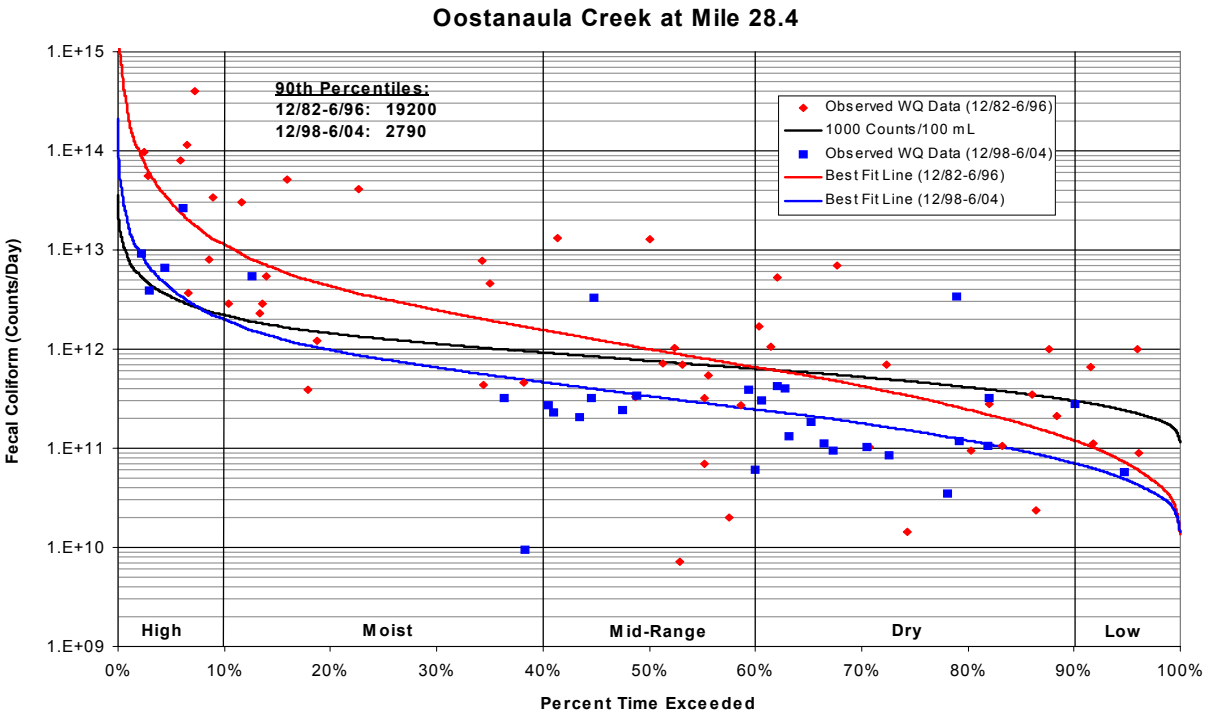


**Figure 15. Oostanaula Creek TMDL implementation effectiveness (LDC regression analysis).**

## 10.0 PUBLIC PARTICIPATION

In accordance with 40 CFR §130.7, the proposed pathogen TMDLs for the Powell River Watershed were placed on Public Notice for a 35-day period and comments solicited. Steps that were taken in this regard include:

1) Notice of the proposed TMDLs was posted on the Tennessee Department of Environment and Conservation website. The announcement invited public and stakeholder comment and provided a link to a downloadable version of the TMDL document.

2) Notice of the availability of the proposed TMDLs (similar to the website announcement) was included in one of the NPDES permit Public Notice mailings which is sent to approximately 90 interested persons or groups who have requested this information.

3) Letters were sent to WWTFs located in E. coli-impaired subwatersheds or drainage areas in the Powell River Watershed, permitted to discharge treated effluent containing pathogens, advising them of the proposed TMDLs and their availability on the TDEC website. The letters also stated that a copy of the draft TMDL document would be provided on request. A letter was sent to the following facilities:

   Lincoln Memorial University (TN0023256)
   Claiborne County Utility District (TN0024791)
   Powell Valley Elementary School (TN0059978)
   Cumberland Gap STP (TN0060747)

4) A draft copy of the proposed TMDL was sent to those MS4s that were wholly or partially located in E. coli-impaired subwatersheds. A draft copy was sent to the following entities:

   Tennessee Dept. of Transportation (TNS077585)

5) A letter was sent to water quality partners in the Powell River Watershed advising them of the proposed pathogen TMDLs and their availability on the TDEC website. The letter also stated that a written copy of the draft TMDL document would be provided upon request. A letter was sent to the following partners:

   Clinch-Powell RC&D Council
   Natural Resources Conservation Service
   Tennessee Valley Authority
   Tennessee Department of Agriculture
   Tennessee Wildlife Resources Agency
   The Nature Conservancy

No comments were received during the public notice period.

## 11.0 FURTHER INFORMATION

Further information concerning Tennessee's TMDL program can be found on the Internet at the Tennessee Department of Environment and Conservation website:

http://www.state.tn.us/environment/wpc/tmdl/

Technical questions regarding this TMDL should be directed to the following members of the Division of Water Pollution Control staff:

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# APPENDIX A

## Land Use Distribution in the Powell River Watershed

**Table A-1. MRLC Land Use Distribution of Powell River Subwatersheds**

| Land Use | HUC-12 Subwatershed (06010206__) or Drainage Area | | | | | |
|---|---|---|---|---|---|---|
| | 0205 (Mulberry Creek) | | Little Creek DA | | 0302 (Russell Creek) | |
| | [acres] | [%] | [acres] | [%] | [acres] | [%] |
| Deciduous Forest | 6,912.5 | 46.7 | 3,104.9 | 68.2 | 3,587.2 | 31.0 |
| Emergent Herbaceous Wetlands | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Evergreen Forest | 1,707.3 | 11.5 | 302.9 | 6.7 | 1,625.7 | 14.0 |
| High Intensity Commercial/ Industrial/Transp. | 7.6 | 0.1 | 1.8 | 0.0 | 185.9 | 1.6 |
| High Intensity Residential | 0.0 | 0.0 | 0.0 | 0.0 | 36.9 | 0.3 |
| Low Intensity Residential | 6.2 | 0.0 | 2.4 | 0.1 | 183.7 | 1.6 |
| Mixed Forest | 1,915.5 | 12.9 | 604.2 | 13.3 | 2,142.6 | 18.5 |
| Open Water | 0.0 | 0.0 | 0.0 | 0.0 | 4.9 | 0.0 |
| Other Grasses (Urban/recreation; e.g. parks) | 0.0 | 0.0 | 0.0 | 0.0 | 231.3 | 2.0 |
| Pasture/Hay | 4,119.2 | 27.8 | 538.0 | 11.8 | 3,256.8 | 28.1 |
| Quarries/Strip Mines/Gravel Pits | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Row Crops | 125.2 | 0.8 | 0.0 | 0.0 | 304.7 | 2.6 |
| Transitional | 0.0 | 0.0 | 0.0 | 0.0 | 12.5 | 0.1 |
| Woody Wetlands | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total | 14,793.5 | 100.0 | 4,554.2 | 100.0 | 11,592.1 | 100.0 |

**Table A-1 (cont'd). MRLC Land Use Distribution of Powell River Subwatersheds**

| Land Use | HUC-12 Subwatershed (06010206__) or Drainage Area | | | | | |
|---|---|---|---|---|---|---|
| | Gap Creek DA | | Cawood Branch DA | | Davis Creek DA | |
| | [acres] | [%] | [acres] | [%] | [acres] | [%] |
| Deciduous Forest | 4,557.5 | 47.8 | 1,181.4 | 55.2 | 8,612.0 | 41.6 |
| Emergent Herbaceous Wetlands | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Evergreen Forest | 905.8 | 9.5 | 153.5 | 7.2 | 1,811.6 | 8.7 |
| High Intensity Commercial/ Industrial/Transp. | 156.1 | 1.6 | 16.5 | 0.8 | 76.9 | 0.4 |
| High Intensity Residential | 18.7 | 0.2 | 0.4 | 0.0 | 0.4 | 0.0 |
| Low Intensity Residential | 129.9 | 1.4 | 1.1 | 0.1 | 17.8 | 0.1 |
| Mixed Forest | 1,302.3 | 13.7 | 253.1 | 11.8 | 2,719.4 | 13.1 |
| Open Water | 12.2 | 0.1 | 0.0 | 0.0 | 2.2 | 0.0 |
| Other Grasses (Urban/recreation; e.g. parks) | 202.6 | 2.1 | 0.0 | 0.0 | 34.0 | 0.2 |
| Pasture/Hay | 2,009.1 | 21.1 | 482.2 | 22.5 | 6,302.9 | 30.4 |
| Quarries/Strip Mines/Gravel Pits | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Row Crops | 228.0 | 2.4 | 52.7 | 2.5 | 1,058.4 | 5.1 |
| Transitional | 14.9 | 0.2 | 0.0 | 0.0 | 68.7 | 0.3 |
| Woody Wetlands | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total | 9,537.2 | 100.0 | 2,140.8 | 100.0 | 20,704.5 | 100.0 |

**Table A-1 (cont'd). MRLC Land Use Distribution of Powell River Subwatersheds**

| Land Use | HUC-12 Subwatershed (06010206__) or Drainage Area | |
|---|---|---|
| | Russell Branch DA | |
| | [acres] | [%] |
| Deciduous Forest | 996.8 | 59.3 |
| Emergent Herbaceous Wetlands | 0.0 | 0.0 |
| Evergreen Forest | 56.7 | 3.4 |
| High Intensity Commercial/ Industrial/Transp. | 2.0 | 0.1 |
| High Intensity Residential | 0.0 | 0.0 |
| Low Intensity Residential | 0.4 | 0.0 |
| Mixed Forest | 146.8 | 8.7 |
| Open Water | 0.0 | 0.0 |
| Other Grasses (Urban/recreation; e.g. parks) | 0.0 | 0.0 |
| Pasture/Hay | 423.2 | 25.2 |
| Quarries/Strip Mines/Gravel Pits | 0.0 | 0.0 |
| Row Crops | 54.5 | 3.2 |
| Transitional | 0.0 | 0.0 |
| Woody Wetlands | 0.0 | 0.0 |
| Total | 1,680.4 | 100.0 |

# APPENDIX B

## Water Quality Monitoring Data

There are a number of water quality monitoring stations that provide data for waterbodies identified as impaired for pathogens in the Powell River Watershed. The location of these monitoring stations is shown in Figure 5. Monitoring data recorded by TDEC at these stations are tabulated in Table B-1.

**Table B-1. TDEC Water Quality Monitoring Data – Powell River Subwatersheds**

| Monitoring Station | Date | E. Coli [cts./100 mL] |
|---|---|---|
| **CAWOO000.2CL** | 1/20/00 | 2419 |
| | 2/24/00 | 236 |
| | 4/26/00 | 517 |
| | 5/22/00 | 1414 |
| | 6/6/00 | 1300 |
| | 7/17/00 | 196 |
| | 8/9/00 | 461 |
| | 9/26/00 | 613 |
| | 10/25/00 | 1553 |
| | 11/14/00 | 866 |
| | 12/11/00 | 166 |
| **DAVIS011.6CL** | 1/18/00 | >2419 |
| | 2/22/00 | 411 |
| | 3/28/00 | 387 |
| | 4/24/00 | 816 |
| | 5/22/00 | 228 |
| | 6/5/00 | 1120 |
| | 7/17/00 | 205 |
| | 8/9/00 | 687 |
| | 9/26/00 | 131 |
| | 10/25/00 | 225 |
| | 11/14/00 | 225 |
| | 12/11/00 | 39 |
| **DAVIS014.6CL** | 1/18/00 | >2419 |
| | 2/22/00 | 1986 |
| | 3/28/00 | 313 |
| | 4/24/00 | 1300 |
| | 6/5/00 | 649 |
| | 7/18/00 | 261 |
| | 8/9/00 | 613 |
| | 9/26/00 | 1190 |
| | 10/24/00 | 980 |

**Table B-1 (Cont.). TDEC Water Quality Monitoring Data – Powell River Subwatersheds**

| Monitoring Station | Date | E. Coli [cts./100 mL] |
|---|---|---|
| **DAVIS014.6CL (cont'd)** | 11/14/00 | 411 |
| | 12/12/00 | 285 |
| | 5/24/00 | 2419 |
| | 7/8/04 | >2419 |
| | 7/19/04 | >2419 |
| | 8/2/04 | 1986 |
| | 8/17/04 | 1733 |
| | 8/19/04 | 770 |
| | 8/26/04 | 1120 |
| | 9/2/04 | 613 |
| | 9/21/04 | 687 |
| | 10/4/04 | 687 |
| | 10/6/04 | 411 |
| | 10/11/04 | 866 |
| **DAVIS016.2CL** | 1/20/00 | 846 |
| | 2/24/00 | 517 |
| | 3/28/00 | 1120 |
| | 4/26/00 | >2419 |
| | 5/24/00 | 866 |
| | 6/7/00 | 411 |
| | 7/18/00 | 2400 |
| | 8/8/00 | 4135 |
| | 9/27/00 | 970 |
| | 10/25/00 | 1560 |
| | 11/14/00 | 921 |
| | 12/12/00 | 1046 |
| **DAVIS018.0CL** | 4/25/00 | >2419 |
| | 10/24/00 | 108 |
| **DAVIS018.1CL** | 1/19/00 | >2419 |
| | 2/23/00 | 816 |
| | 3/23/00 | >2419 |
| | 5/23/00 | 613 |
| | 6/7/00 | 687 |
| | 7/18/00 | 980 |
| | 8/8/00 | 1749 |

**Table B-1 (Cont.). TDEC Water Quality Monitoring Data – Powell River Subwatersheds**

| Monitoring Station | Date | E. Coli [cts./100 mL] |
|---|---|---|
| **DAVIS018.1CL (cont'd)** | 9/27/00 | 310 |
| | 12/12/00 | 816 |
| **DAVIS020.5CL** | 1/19/00 | 980 |
| | 2/23/00 | 219 |
| | 3/28/00 | >2419 |
| | 4/25/00 | >2419 |
| | 5/23/00 | 980 |
| | 6/6/00 | 387 |
| | 7/18/00 | 300 |
| | 8/8/00 | 488 |
| | 9/27/00 | 118 |
| | 10/24/00 | 29 |
| | 11/15/00 | 73 |
| | 12/12/00 | 111 |
| **DAVIS022.6CL** | 1/19/00 | 1733 |
| | 2/23/00 | 77 |
| | 3/28/00 | 1120 |
| | 4/25/00 | >2419 |
| | 5/23/00 | 461 |
| | 6/6/00 | 1986 |
| | 7/18/00 | 222 |
| | 8/8/00 | 1200 |
| | 9/27/00 | 108 |
| | 10/24/00 | 479 |
| | 11/15/00 | 378 |
| | 12/12/00 | 260 |
| **GAP004.3CL** | 7/8/04 | 179 |
| | 7/19/04 | 649 |
| | 8/2/04 | 770 |
| | 8/17/04 | 238 |
| | 8/19/04 | 613 |
| | 8/26/04 | 299 |
| | 9/2/04 | 276 |
| | 9/21/04 | 186 |
| | 10/4/04 | 99 |

**Table B-1 (Cont.). TDEC Water Quality Monitoring Data – Powell River Subwatersheds**

| Monitoring Station | Date | E. Coli [cts./100 mL] |
|---|---|---|
| **GAP004.3CL (cont'd)** | 10/6/04 | 228 |
| | 10/11/04 | 186 |
| **LITTL000.1CL** | 10/23/02 | 238 |
| | 11/5/02 | 416 |
| | 12/3/02 | 43 |
| | 7/8/04 | 173 |
| | 7/19/04 | 201 |
| | 8/2/04 | 548 |
| | 8/17/04 | 197 |
| | 8/19/04 | 461 |
| | 8/26/04 | 299 |
| | 9/2/04 | 161 |
| | 9/21/04 | 687 |
| | 10/4/04 | 249 |
| | 10/6/04 | 86 |
| | 10/11/04 | 121 |
| **LMULB000.6HK** | 3/23/00 | >2419 |
| | 6/21/00 | >2419 |
| | 12/6/00 | 687 |
| | 7/29/04 | 1986 |
| | 8/25/04 | 32550 |
| | 9/14/04 | 2620 |
| | 10/26/04 | 816 |
| | 11/30/04 | 727 |
| | 12/21/04 | 4000 |
| | 1/25/05 | 13540 |
| | 2/24/05 | 1310 |
| | 3/17/05 | 6380 |
| | 4/26/05 | 1100 |
| | 5/18/05 | 2160 |
| | 6/22/05 | 1730 |
| **MULBE000.3HK** | 10/23/02 | 365 |
| | 11/6/02 | >2419 |
| | 12/3/02 | 461 |

**Table B-1 (Cont.). TDEC Water Quality Monitoring Data – Powell River Subwatersheds**

| Monitoring Station | Date | E. Coli [cts./100 mL] |
|---|---|---|
| **MULBE000.7HK** | 3/23/00 | 68 |
| | 6/21/00 | >2419 |
| | 12/6/00 | 172 |
| | 7/29/04 | 687 |
| | 8/25/04 | 2110 |
| | 9/14/04 | 649 |
| | 10/26/04 | 770 |
| | 11/30/04 | 20460 |
| | 12/21/04 | 866 |
| | 1/25/05 | 365 |
| | 2/24/05 | 461 |
| | 3/17/05 | 410 |
| | 4/26/05 | 1733 |
| | 5/18/05 | 1613 |
| | 6/22/05 | 630 |
| **RUSSE000.3CL** | 1/20/00 | 1120 |
| | 2/24/00 | 260 |
| | 3/28/00 | 1553 |
| | 4/25/00 | >2419 |
| | 5/23/00 | >2419 |
| | 6/6/00 | 1733 |
| | 7/18/00 | 2280 |
| | 8/8/00 | 6086 |
| | 9/26/00 | 1350 |
| | 10/25/00 | 1733 |
| | 11/15/00 | 1733 |
| | 12/12/00 | 2419 |
| **RUSSE000.5CL** | 6/22/04 | 1733 |
| | 6/28/04 | 260 |
| | 7/1/04 | 248 |
| | 7/7/04 | 345 |
| | 7/14/04 | 579 |
| | 7/20/04 | 157 |
| | 7/22/04 | 727 |
| | 8/3/04 | 687 |

**Table B-1 (Cont.). TDEC Water Quality Monitoring Data – Powell River Subwatersheds**

| Monitoring Station | Date | E. Coli [cts./100 mL] |
|---|---|---|
| **RUSSE000.5CL (cont'd)** | 8/16/04 | 219 |
| | 9/2/04 | 225 |
| | 9/23/04 | 192 |
| | 9/29/04 | 548 |

# APPENDIX C

## Load Duration Curve Development
## and
## Determination of Daily Loading

The TMDL process quantifies the amount of a pollutant that can be assimilated in a waterbody, identifies the sources of the pollutant, and recommends regulatory or other actions to be taken to achieve compliance with applicable water quality standards based on the relationship between pollution sources and in-stream water quality conditions. A TMDL can be expressed as the sum of all point source loads (Waste Load Allocations), nonpoint source loads (Load Allocations), and an appropriate margin of safety (MOS) that takes into account any uncertainty concerning the relationship between effluent limitations and water quality:

$$TMDL = \Sigma\ WLAs + \Sigma\ LAs + MOS$$

The objective of a TMDL is to allocate loads among all of the known pollutant sources throughout a watershed so that appropriate control measures can be implemented and water quality standards achieved. 40 CFR §130.2 (i) (http://www.epa.gov/epacfr40/chapt-I.info/chi-toc.htm ) states that TMDLs can be expressed in terms of mass per time, toxicity, or other appropriate measure.

## C.1 Development of TMDLs

E. coli TMDLs, WLAs, and LAs were developed for impaired subwatersheds and drainage areas in the Powell River Watershed using Load Duration Curves (LDCs). ). Daily loads for TMDLs, WLAs, and LAs are expressed as a function of daily mean in-stream flow (daily loading function).

### C.1.1 Development of Flow Duration Curves

A flow duration curve is a cumulative frequency graph, constructed from historic flow data at a particular location, that represents the percentage of time a particular flow rate is equaled or exceeded. Flow duration curves are developed for a waterbody from daily discharges of flow over an extended period of record. In general, there is a higher level of confidence that curves derived from data over a long period of record correctly represent the entire range of flow. The preferred method of flow duration curve computation uses daily mean data from U.S. Geological Survey (USGS) continuous-record stations (http://waterdata.usgs.gov/tn/nwis/sw ) located on the waterbody of interest. For ungaged streams, alternative methods must be used to estimate daily mean flow. These include: 1) regression equations (using drainage area as the independent variable) developed from continuous record stations in the same ecoregion; 2) drainage area extrapolation of data from a nearby continuous-record station of similar size and topography; and 3) calculation of daily mean flow using a dynamic computer model, such as the Loading Simulation Program C++ (LSPC).

Flow duration curves for impaired waterbodies in the Powell River Watershed were derived from LSPC hydrologic simulations based on parameters derived from calibrations at USGS Station No. 03491000 (see Appendix D for details of calibration). For example, a flow-duration curve for Davis Creek at RM 14.6 was constructed using simulated daily mean flow for the period from 10/1/97 through 9/30/07 (RM 14.6 corresponds to the location of monitoring station DAVIS014.6CL). This flow duration curve is shown in Figure C-1 and represents the cumulative distribution of daily discharges arranged to show percentage of time specific flows were exceeded during the period of record (the highest daily mean flow during this period is exceeded 0% of the time and the lowest daily mean flow is equaled or exceeded 100% of the time). Flow duration curves for other impaired waterbodies were derived using a similar procedure.

### C.1.2 Development of Load Duration Curves and TMDLs

When a water quality target concentration is applied to the flow duration curve, the resulting load duration curve (LDC) represents the allowable pollutant loading in a waterbody over the entire range of flow. Pollutant monitoring data, plotted on the LDC, provides a visual depiction of stream water quality as well as the frequency and magnitude of any exceedances. Load duration curve intervals can be grouped into several broad categories or zones, in order to provide additional insight about conditions and patterns associated with the impairment. For example, the duration curve could be divided into five zones: high flows (exceeded 0-10% of the time), moist conditions (10-40%), median or mid-range flows (40-60%), dry conditions (60-90%), and low flows (90-100%). Impairments observed in the low flow zone typically indicate the influence of point sources, while those further left on the LDC (representing zones of higher flow) generally reflect potential nonpoint source contributions (Stiles, 2003).

E. coli load duration curves for impaired waterbodies in the Powell River Watershed were developed from the flow duration curves developed in Section C.1.1, E. coli target concentrations, and available water quality monitoring data. Load duration curves and required load reductions were developed using the following procedure (Davis Creek is shown as an example):

1. A target load-duration curve (LDC) was generated for Davis Creek by applying the E. coli target concentration of 941 CFU/100 mL to each of the ranked flows used to generate the flow duration curve (ref.: Section D.1) and plotting the results. The E. coli target maximum load corresponding to each ranked daily mean flow is:

   $(\text{Target Load})_{\text{Davis Creek}}$ = (941 CFU/100 mL) x (Q) x (UCF)

   where: Target Load = TMDL (CFU/day)
   Q = daily instream mean flow
   UCF = the required unit conversion factor

   **TMDL = ($2.30 \times 10^{10}$) x (Q) CFU/day**

2. Daily loads were calculated for each of the water quality samples collected at monitoring station DAVIS014.6CL (ref.: Table B-1) by multiplying the sample concentration by the daily mean flow for the sampling date and the required unit conversion factor. DAVIS014.6CL was selected for LDC analysis because it has multiple exceedances of the target concentration.

   *Note: In order to be consistent for all analyses, the derived daily mean flow was used to compute sampling data loads, even if measured ("instantaneous") flow data was available for some sampling dates.*

   Example – 9/26/00 sampling event:
   *Modelled Flow = 2.76 cfs*
   *Concentration = 1190 CFU/100 mL*
   *Daily Load = $8.04 \times 10^{10}$ CFU/day*

3. Using the flow duration curves developed in C.1.1, the "percent of days the flow was exceeded" (PDFE) was determined for each sampling event. Each sample load was then plotted on the load duration curves developed in Step 1 according to the PDFE. The resulting E. coli load duration curve for is shown in Figure C-2.

LDCs of other impaired waterbodies were derived in a similar manner and are shown in Appendix E.

## C.2 Development of WLAs & LAs

As previously discussed, a TMDL can be expressed as the sum of all point source loads (WLAs), nonpoint source loads (LAs), and an appropriate margin of safety (MOS) that takes into account any uncertainty concerning the relationship between effluent limitations and water quality:

$$TMDL = \Sigma\ WLAs + \Sigma\ LAs + MOS$$

Expanding the terms:

$$TMDL = [\sum WLAs]_{WWTF} + [\sum WLAs]_{MS4} + [\sum WLAs]_{CAFO} + [\sum LAs]_{DS} + [\sum LAs]_{SW} + MOS$$

For E. coli TMDLs in each impaired subwatershed or drainage area, WLA terms include:

- $[\sum WLAs]_{WWTF}$ is the allowable load associated with discharges of NPDES permitted WWTFs located in impaired subwatersheds or drainage areas. Since NPDES permits for these facilities specify that treated wastewater must meet in-stream water quality standards at the point of discharge, no additional load reduction is required. WLAs for WWTFs are calculated from the facility design flow and the Monthly Average permit limit.
- $[\sum WLAs]_{CAFO}$ is the allowable load for all CAFOs in an impaired subwatershed or drainage area. All wastewater discharges from a CAFO to waters of the state of Tennessee are prohibited, except when either chronic or catastrophic rainfall events cause an overflow of process wastewater from a facility properly designed, constructed, maintained, and operated to contain:
  - All process wastewater resulting from the operation of the CAFO (such as wash water, parlor water, watering system overflow, etc.); plus,
  - All runoff from a 25-year, 24-hour rainfall event for the existing CAFO or new dairy or cattle CAFOs; or all runoff from a 100-year, 24-hour rainfall event for a new swine or poultry CAFO.

  Therefore, a WLA of zero has been assigned to this class of facilities.
- $[\sum WLAs]_{MS4}$ is the allowable E. coli load for discharges from MS4s. E. coli loading from MS4s is the result of buildup/wash-off processes associated with storm events.

LA terms include:

- $[\sum LAs]_{DS}$ is the allowable E. coli load from "other direct sources". These sources include leaking septic systems, illicit discharges, and animals access to streams. The LA specified for all sources of this type is zero CFU/day (or to the maximum extent feasible).

- $[\sum LAs]_{SW}$ represents the allowable E. coli loading from nonpoint sources indirectly going to surface waters from all land use areas (except areas covered by a MS4 permit) as a result of the buildup/wash-off processes associated with storm events (i.e., precipitation induced).

Since $[\sum WLAs]_{CAFO} = 0$ and $[\sum LAs]_{DS} = 0$, the expression relating TMDLs to precipitation-based point and nonpoint sources may be simplified to:

$$TMDL - MOS = [WLAs]_{WWTF} + [\sum WLAs]_{MS4} + [\sum LAs]_{SW}$$

As stated in Section 8.4, an explicit MOS, equal to 10% of the E. coli water quality targets (ref.: Section 5.0), was utilized for determination of the percent load reductions necessary to achieve and WLAs and LAs:

Instantaneous Maximum (lake, reservoir, State Scenic River, Exceptional Tennessee Waters):

Target – MOS = (487 CFU/100 ml) – 0.1(487 CFU/100 ml)

Target – MOS = 438 CFU/100 ml

Instantaneous Maximum (other):

Target – MOS = (941 CFU/100 ml) – 0.1(941 CFU/100 ml)

Target – MOS = 847 CFU/100 ml

30-Day Geometric Mean: Target – MOS = (126 CFU/100 ml) – 0.1(126 CFU/100 ml)

Target – MOS = 113 CFU/100 ml

### C.2.1 Daily Load Calculation

Since WWTFs discharge must comply with instream water quality criteria (TMDL target) at the point of discharge, WLAs for WWTFs are expressed as a constant term. In addition, WLAs for MS4s and LAs for precipitation-based nonpoint sources are equal on a per unit area basis and may be expressed as the daily allowable load per unit area (acre) resulting from a decrease in in-stream E. coli concentrations to TMDL target values minus MOS:

$$WLA[MS4] = LA = \{TMDL - MOS - WLA[WWTFs]\} / DA$$

where: DA = waterbody drainage area (acres)

Using Davis Creek as an example:

$$TMDL_{Davis\ Creek} = (941\ CFU/100\ mL) \times (Q) \times (UCF)$$

$$= 2.30 \times 10^{10} \times Q$$

$MOS_{Davis\ Creek}$ = TMDL x 0.10 = $2.30 \times 10^9$ x Q

**MOS = ($2.30 \times 10^9$) x (Q) CFU/day**

$WLA[MS4]_{Davis\ Creek}$ = $LA_{Davis\ Creek}$

= {TMDL – MOS – WLA[WWTFs]} / DA

= {($2.30 \times 10^{10}$ x Q) – ($2.30 \times 10^9$ x Q) – (0)} / ($1.57 \times 10^4$)

**WLA[MS4] = LA = [$1.323 \times 10^6$ x Q]**

TMDLs, WLAs, & LAs for other impaired subwatersheds and drainage areas were derived in a similar manner and are summarized in Table C-1.

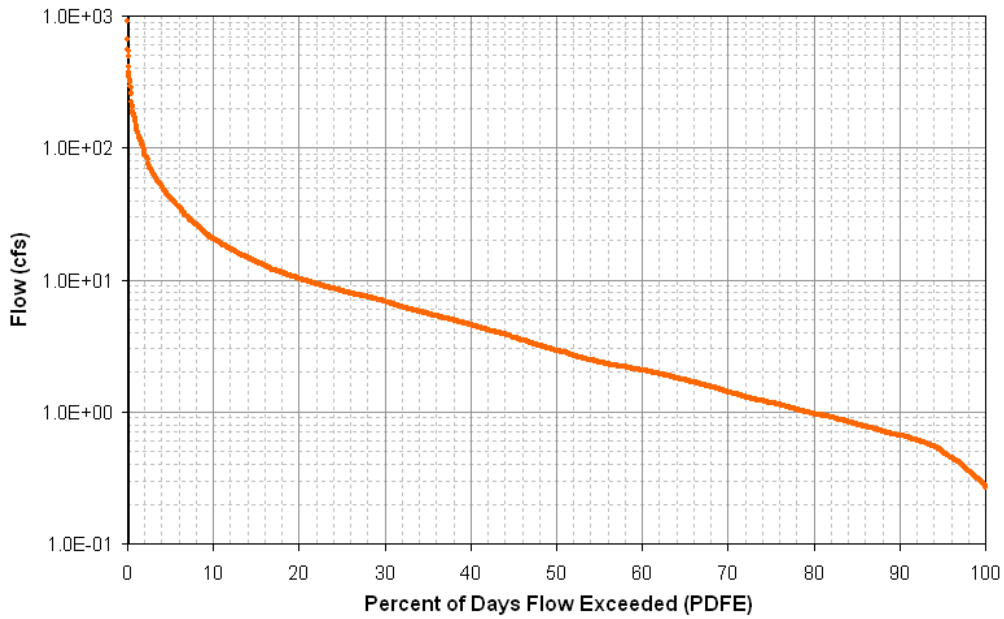


**Figure C-1. Flow Duration Curve for Davis Creek at Mile 14.6**

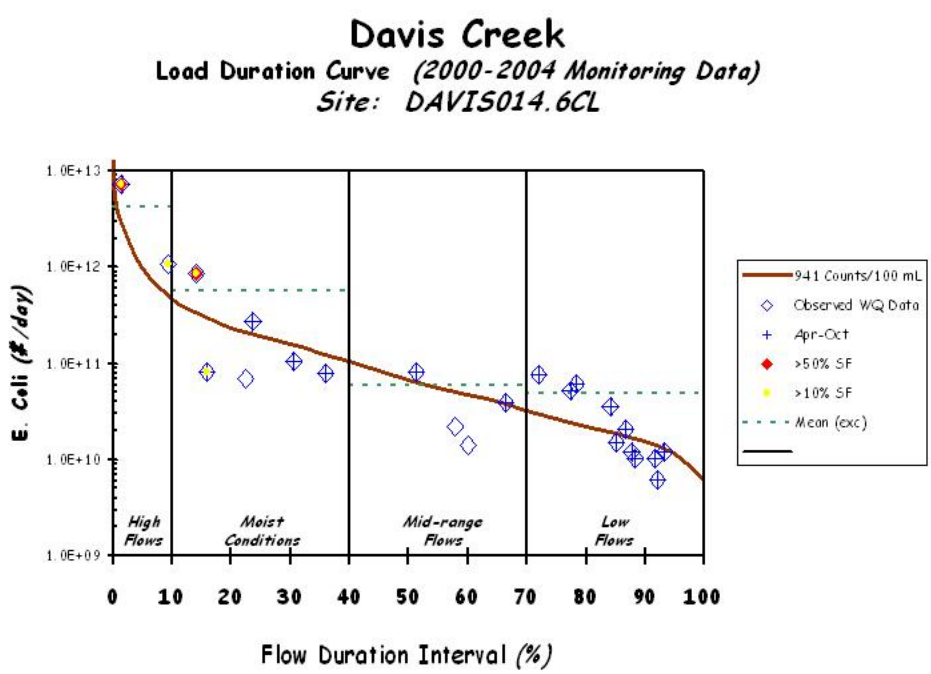


**Figure C-2. E. Coli Load Duration Curve for Davis Creek at Mile 14.6**

**Table C-1. TMDLs, WLAs, & LAs for Impaired Waterbodies in the Powell River Watershed (HUC 06010206)**

| HUC-12 Subwatershed (06010206__) or Drainage Area (DA) | Impaired Waterbody Name | Impaired Waterbody ID | TMDL | MOS | WLAs | | | | LAs |
|---|---|---|---|---|---|---|---|---|---|
| | | | | | WWTFs [a] | Collection Systems | CAFOs | MS4s [b] | |
| | | | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/day] | [CFU/d/ac] | [CFU/d/ac] |
| 0205 | Little Mulberry Creek | TN06010104007 – 0710 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $7.877 \times 10^{6} \times Q$ |
| | Mulberry Creek | TN06010104007 – 0700 | $1.20 \times 10^{10} \times Q$ | $1.20 \times 10^{9} \times Q$ | NA | NA | NA | NA | $7.425 \times 10^{5} \times Q$ |
| 0301 (DA) | Little Creek | TN06010104007 – 0100 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $4.545 \times 10^{6} \times Q$ |
| 0302 | Russell Creek | TN06010104008 – 1000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | $2.315 \times 10^{10}$ | 0 | NA | NA | $1.913 \times 10^{6} \times Q - 2.140 \times 10^{6}$ |
| 0304 (DA) | Gap Creek | TN06010104006 – 0200 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | $1.843 \times 10^{9}$ | 0 | NA | NA | $2.170 \times 10^{6} \times Q - 1.933 \times 10^{5}$ |
| 0307 (DA) | Cawood Branch | TN06010104026 – 0100 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $9.668 \times 10^{6} \times Q$ |
| 0307 (DA) | Russell Branch | TN06010104026 – 0200 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | NA | NA | $1.232 \times 10^{7} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 1000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $9.998 \times 10^{5} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 2000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $1.607 \times 10^{6} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 3000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $2.777 \times 10^{6} \times Q$ |
| 0307 (DA) | Davis Creek | TN06010104026 – 4000 | $2.30 \times 10^{10} \times Q$ | $2.30 \times 10^{9} \times Q$ | NA | NA | 0 | NA | $1.088 \times 10^{7} \times Q$ |

Notes: NA = Not Applicable.
Q = Mean Daily In-stream Flow (cfs).

a. WLAs for WWTFs are expressed as E. coli loads (CFU/day). All current and future WWTFs must meet water quality standards as specified in their NPDES permit.

b. Applies to any MS4 discharge loading in the subwatershed. Future MS4s will be assigned waste load allocations (WLAs) consistent with load allocations (LAs) assigned to precipitation induced nonpoint sources.

# APPENDIX D

## Hydrodynamic Modeling Methodology

## HYDRODYNAMIC MODELING METHODOLOGY

### D.1 Model Selection

The Loading Simulation Program C++ (LSPC) was selected for flow simulation of pathogen-impaired waters in the subwatersheds of the Powell River Watershed. LSPC is a watershed model capable of performing flow routing through stream reaches. LSPC is a dynamic watershed model based on the Hydrologic Simulation Program - Fortran (HSPF)

### D.2 Model Set Up

The Powell River Watershed was delineated into subwatersheds in order to facilitate model hydrologic calibration. Boundaries were constructed so that subwatershed “pour points” coincided with HUC-12 delineations, 303(d)-listed waterbodies, and water quality monitoring stations. Watershed delineation was based on the NHD stream coverage and Digital Elevation Model (DEM) data. This discretization facilitates simulation of daily flows at water quality monitoring stations.

Several computer-based tools were utilized to generate input data for the LSPC model. The Watershed Characterization System (WCS), a geographic information system (GIS) tool, was used to display, analyze, and compile available information to support hydrology model simulations for selected subwatersheds. This information includes land use categories, point source dischargers, soil types and characteristics, population data (human and livestock), and stream characteristics.

An important factor influencing model results is the precipitation data contained in the meteorological data files used in these simulations. Weather data from multiple meteorological stations were available for the time period from January 1970 through September 2007. Meteorological data for a selected 11-year period were used for all simulations. The first year of this period was used for model stabilization with simulation data from the subsequent 10-year period (10/1/97 – 9/30/07) used for TMDL analysis.

### D.3 Model Calibration

Hydrologic calibration of the watershed model involves comparison of simulated streamflow to historic streamflow data from U. S. Geological Survey (USGS) stream gaging stations for the same period of time. A USGS continuous record station located near the Powell River Watershed with a sufficiently long and recent historical record was selected as the basis of the hydrology calibration. The USGS station was selected based on similarity of drainage area, Level IV ecoregion, land use, and topography. The calibration involved comparison of simulated and observed hydrographs until statistical stream volumes and flows were within acceptable ranges as reported in the literature (Lumb, et al., 1994).

Initial values for hydrologic variables were taken from an EPA developed default data set. During the calibration process, model parameters were adjusted within reasonable constraints until acceptable agreement was achieved between simulated and observed streamflow. Model parameters adjusted include: evapotranspiration, infiltration, upper and lower zone storage, groundwater storage, recession, losses to the deep groundwater system, and interflow discharge.

The results of the hydrologic calibration for Big Creek near Rogersville, USGS Station 03491000, drainage area 46.5 square miles, are shown in Table D-1 and Figures D-1 and D-2.

**Table D-1. Hydrologic Calibration Summary: Big Creek near Rogersville (USGS 03491000)**

| | | | | |
|---|---|---|---|---|
| | | | 46.54168002 | |
| | **Simulation Name:** | USGS03491000 | **Simulation Period:** | |
| | | | **Watershed Area (ac):** | 29796.21 |
| | ***Period for Flow Analysis*** | | | |
| | ***Begin Date:*** | 10/01/96 | **Baseflow PERCENTILE:** | 2.5 |
| | ***End Date:*** | 09/30/06 | *Usually 1%-5%* | |
| | Total Simulated In-stream Flow: | **152.74** | Total Observed In-stream Flow: | **147.28** |
| | Total of highest 10% flows: | **85.01** | Total of Observed highest 10% flows: | **75.48** |
| | Total of lowest 50% flows: | **12.46** | Total of Observed Lowest 50% flows: | **12.71** |
| | Simulated Summer Flow Volume ( months 7-9): | **17.06** | Observed Summer Flow Volume (7-9): | **10.40** |
| | Simulated Fall Flow Volume (months 10-12): | **28.32** | Observed Fall Flow Volume (10-12): | **25.19** |
| | Simulated Winter Flow Volume (months 1-3): | **61.91** | Observed Winter Flow Volume (1-3): | **65.58** |
| | Simulated Spring Flow Volume (months 4-6): | **45.44** | Observed Spring Flow Volume (4-6): | **46.11** |
| | Total Simulated Storm Volume: | **145.27** | Total Observed Storm Volume: | **139.74** |
| | Simulated Summer Storm Volume (7-9): | **15.18** | Observed Summer Storm Volume (7-9): | **8.53** |
| | *Errors (Simulated-Observed)* | | *Recommended Criteria* | Last run |
| | Error in total volume: | **3.70** | 10 | |
| | Error in 50% lowest flows: | **-2.03** | 10 | |
| | Error in 10% highest flows: | **12.63** | 15 | |
| *** | Seasonal volume error - Summer: | **63.97** | 30 | |
| | Seasonal volume error - Fall: | **12.42** | 30 | |
| | Seasonal volume error - Winter: | **-5.59** | 30 | |
| | Seasonal volume error - Spring: | **-1.45** | 30 | |
| | Error in storm volumes: | **3.96** | 20 | |
| *** | Error in summer storm volumes: | **78.06** | 50 | |

| **Criteria for Median Monthly Flow Comparisons** | |
|---|---|
| Lower Bound (Percentile): | 25 |
| Upper Bound (Percentile): | 75 |

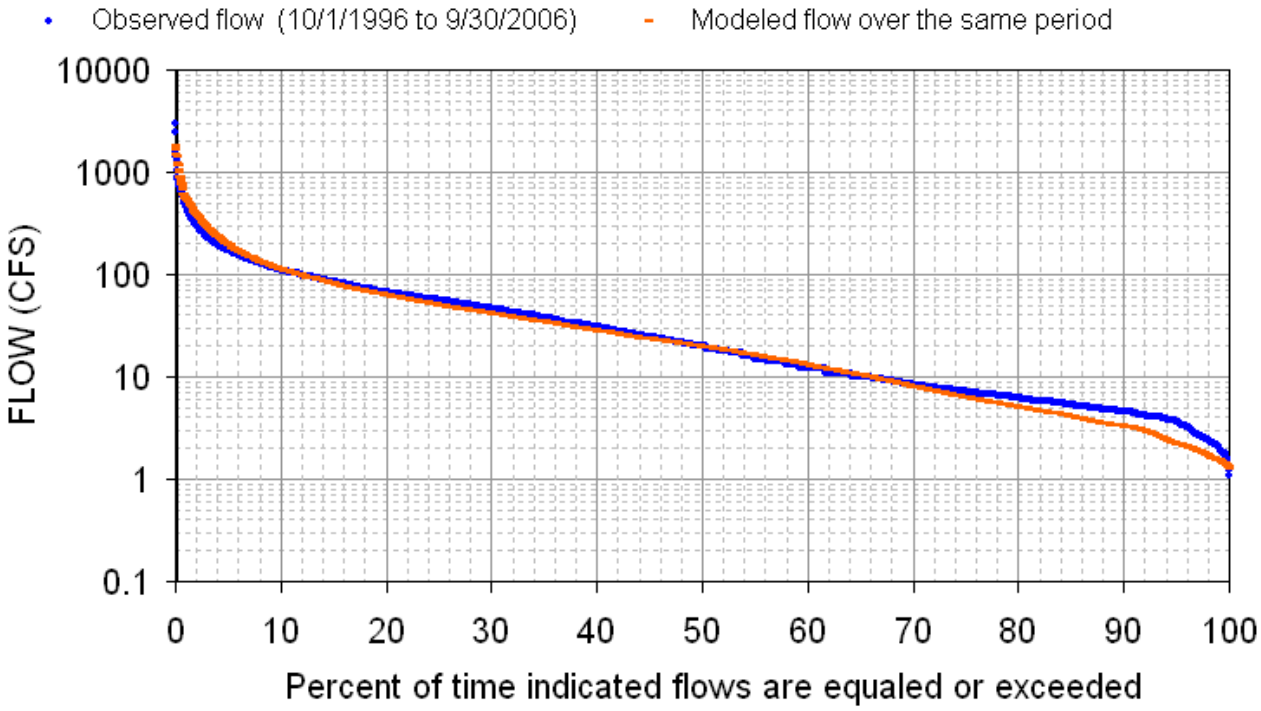


**Figure D-1. Hydrologic Calibration: Big Creek, USGS 03491000 (WYs1997-2006)**

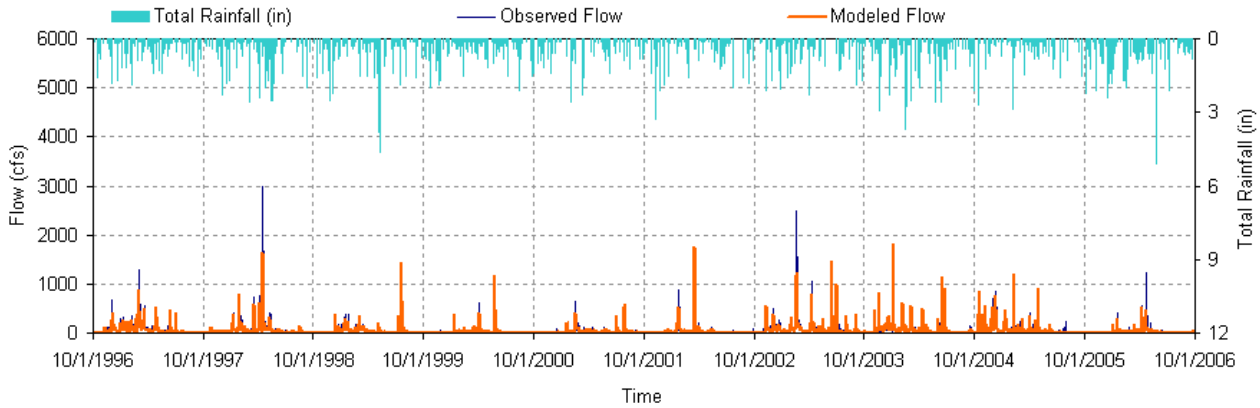


**Figure D-2. 10-Year Hydrologic Comparison: Big Creek, USGS 03491000**

# APPENDIX E

## Source Area Implementation Strategy

All impaired waterbodies and corresponding HUC-12 subwatersheds or drainage areas have been classified according to their respective source area types in Section 9.5, Table 9. The implementation for each area will be prioritized according to the guidance provided in Section 9.5.1 and 9.5.2, with examples provided in Section E.1 and E.2, below. For all impaired waterbodies, the determination of source area types serves to identify the predominant sources contributing to impairment (i.e., those that should be targeted initially for implementation). However, it is not intended to imply that sources in other landuse areas are not contributors to impairment and/or to grant an exemption from addressing other source area contributions with implementation strategies and corresponding load reduction. For mixed use areas, implementation will follow the guidance established for both urban and agricultural areas, at a minimum.

## E.1 Urban Source Areas

For impaired waterbodies and corresponding HUC-12 subwatersheds or drainage areas identified as predominantly urban source area types, the following example for Russell Creek provides guidance for implementation analysis:

The Russell Creek watershed, HUC-12 060102060302, lies near Tazewell. The drainage area for Russell Creek at mile 0.5 is approximately 10,819 acres (16.9 $mi^2$); therefore, four flow zones were used for the duration curve analysis (see Sect. 9.1.1).

*Note: The Final 2008 303(d) List includes Discharges from MS4 Area and Pasture Grazing both as Pollutant Source categories for Russell Creek; therefore, Russell Creek is listed in the Mixed source area type in Section 9.5, Table 9.*

The flow duration curve for Russell Creek at mile 0.5 was constructed using simulated daily mean flow for the period from 10/1/97 through 9/30/07 (mile 0.5 corresponds to the location of monitoring station RUSSE000.5CL). This flow duration curve is shown in Figure E-1 and represents the cumulative distribution of daily discharges arranged to show percentage of time specific flows were exceeded during the period of record. Flow duration curves for other impaired waterbodies were developed using a similar procedure (Appendix C).

The E. coli LDC for Russell Creek at Mile 0.5 (Figure E-2) was analyzed to determine the frequency with which observed daily water quality loads exceed the E. coli target maximum daily loading (941 CFU/100 mL x flow [cfs] x conversion factor) under five flow conditions (low, dry, mid-range, moist, and high). Observation of the plot illustrates that the only exceedance occured under moist flow conditions indicating the Russell Creek watershed may be impacted by non-point-type sources. LDCs for other impaired waterbodies were developed using a similar procedure (Appendix C) and are shown in Figures E-5 thru E-18.

Critical conditions for the Russell Creek watershed (HUC-12 060102060302) occur during moist flow conditions, typically indicative of non-point source contributions (see Table E-3, Section E.4). According to hydrograph separation analysis, the exceedance did not occur during a stormflow event.

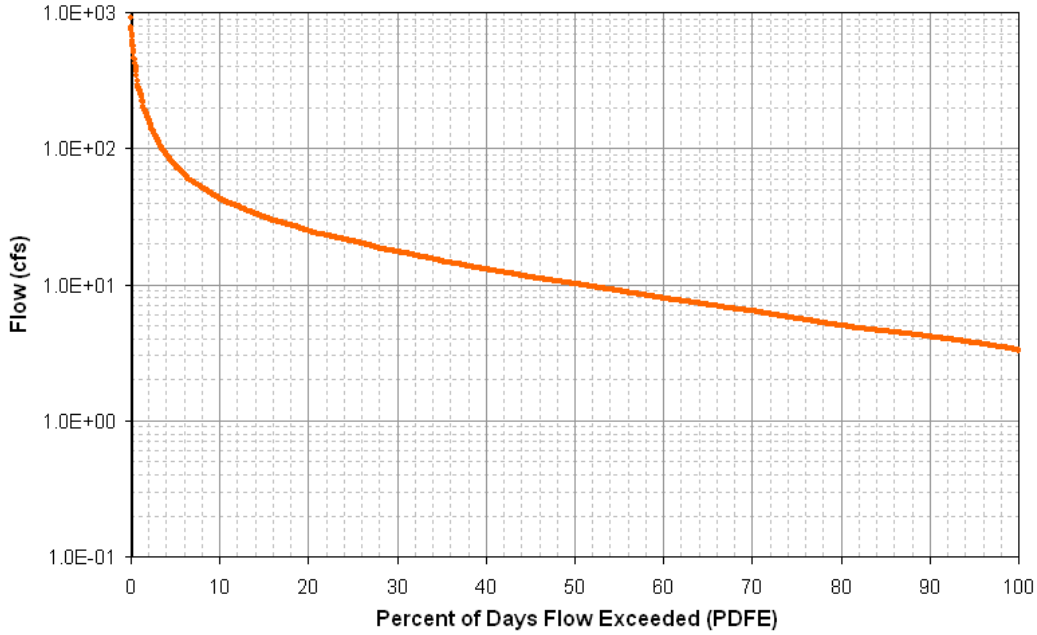


**Figure E-1. Flow Duration Curve for Russell Creek at Mile 0.5**

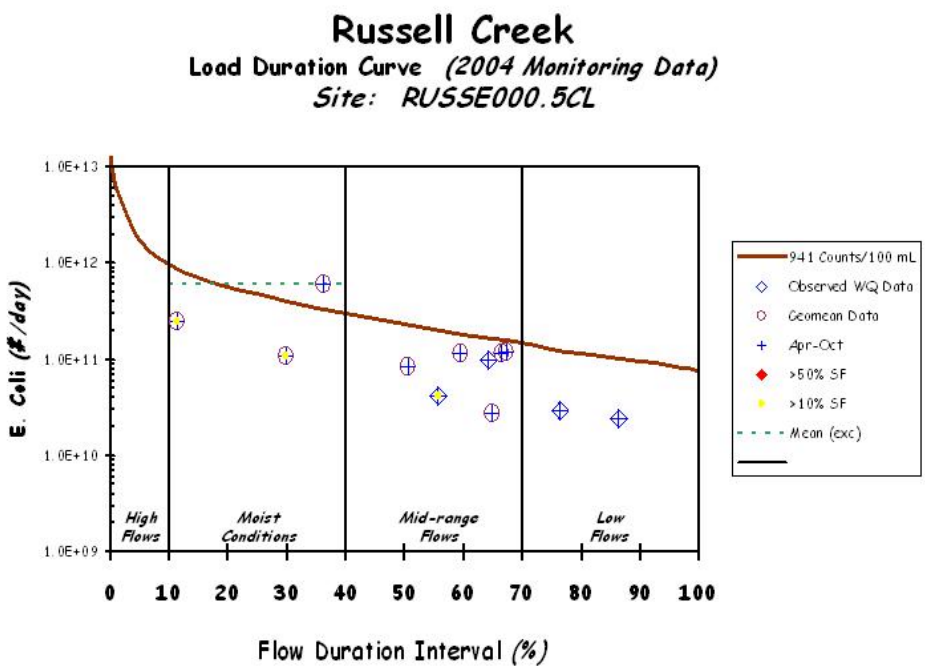


**Figure E-2. E. Coli Load Duration Curve for Russell Creek**

**Table E-1. Load Duration Curve Summary for Implementation Strategies (Example: Russell Creek subwatershed, HUC-12 060102060302) (4 Flow Zones).**

| Hydrologic Condition | | High | Moist | Mid-range* | Low |
|---|---|---|---|---|---|
| % Time Flow Exceeded | | 0-10 | 10-40 | 40-70 | 70-100 |
| Russell Creek (060102060302) | Number of Samples | 0 | 3 | 7 | 2 |
| | % > 941 CFU/100 mL[1] | NR | 33.3 | 0.0 | 0.0 |
| | Load Reduction[2] | NA | 15.2 | NR | NR |
| TMDL (CFU/day) | | 1.740E+12 | 4.793E+11 | 2.075E+11 | 1.044E+11 |
| Margin of Safety (CFU/day) | | 1.740E+11 | 4.793E+10 | 2.075E+10 | 1.044E+10 |
| WLA (WWTFs) (CFU/day) | | 2.315E+10 | 2.315E+10 | 2.315E+10 | 2.315E+10 |
| WLAs (MS4s) (CFU/day/acre)[3] | | NA | NA | NA | NA |
| LA (CFU/day/acre)[3] | | 1.426E+08 | 3.773E+07 | 1.512E+07 | 6.547E+06 |
| Implementation Strategies[4] | | | | | |
| Municipal NPDES | | | L | M | H |
| Stormwater Management | | | H | H | |
| SSO Mitigation | | H | M | L | |
| Collection System Repair | | | H | M | |
| Septic System Repair | | | L | M | M |
| Potential for source area contribution under given flow condition (H: High; M: Medium; L: Low) | | | | | |

* The Moist Conditions zone represents the critical conditions for E. coli loading in the Russell Creek subwatershed.
[1] Tennessee Maximum daily water quality criterion for E. coli.
[2] Reductions (percent) based on mean of observed percent load reductions in range.
[3] LAs and MS4s are expressed as daily load per unit area in order to provide for future changes in the distribution of LAs and MS4s (WLAs).
[4] Watershed-specific Best Management Practices for Urban Source reduction. Actual BMPs applied may vary and should not be limited according to this grouping.

Results indicate the implementation strategy for the Russell Creek watershed will require BMPs targeting point sources (dominant under low flow/baseflow conditions) and non-point sources (dominant under high flow/runoff conditions). Table E-1 presents an allocation table of LDC analysis statistics for Russell Creek E. coli and implementation strategies for each source category covering the entire range of flow (Stiles, 2003). The implementation strategies listed in Table E-1 are a subset of the categories of BMPs and implementation strategies available for application to the Powell River watershed for reduction of E. coli loading and mitigation of water quality impairment from urban sources. Targeted implementation strategies and LDC analysis statistics for other impaired waterbodies and corresponding HUC-12 subwatersheds and drainage areas identified as predominantly urban source area types can be derived from the information and results available in Tables 10 and E-19.

Table E-19 presents LDC analyses (TMDLs, WLAs, LAs, and MOS) and PLRGs for all flow zones for all E. coli impaired waterbodies in the Powell River watershed.

## E.2 Agricultural Source Areas

For impaired waterbodies and corresponding HUC-12 subwatersheds or drainage areas identified as predominantly agricultural source area types, the following example for Davis Creek provides guidance for implementation analysis:

The Davis Creek subwatershed, HUC-12 060102060307, lies in a non-urbanized area of Claiborne county near Norris Lake. The drainage area for Davis Creek at Mile 14.6 is approximately 15,650 acres (24.5 $mi^2$); therefore, four flow zones were used for the duration curve analysis (see Sect. 9.1.1). The landuse for Davis Creek at mile 14.6 is approximately 38% agricultural, with most of the remainder being forested. Urban areas make up less than 0.4% of the total area. Therefore, the predominant landuse type and sources are agricultural.

The flow duration curve for Davis Creek at Mile 14.6 was constructed using simulated daily mean flow for the period from 10/1/97 through 9/30/07. This flow duration curve is shown in Figure E-3 and represents the cumulative distribution of daily discharges arranged to show percentage of time specific flows were exceeded during the period of record. Flow duration curves for other impaired waterbodies were developed using a similar procedure (see Appendix C).

The E. coli LDC for Davis Creek at Mile 14.6 (Figure E-4) was analyzed to determine the frequency with which observed daily water quality loads exceed the E. coli target maximum daily loading (941 CFU/100 mL x flow [cfs] x conversion factor) under four flow conditions (low, mid-range, moist, and high). Observation of the plot illustrates that exceedances occur under most flow zones indicating the Davis Creek watershed is impacted by point and non-point-type sources. LDCs for other impaired waterbodies were developed using a similar procedure (Appendix C) and are shown in Figures E-5 thru E-18.

Critical conditions for the Davis Creek HUC-12 occur during low flow conditions, typically indicative of point source contributions (see Table E-3, Section E.4). Exceedances of the E. coli water quality standard are fairly well distributed across the full range of flows and flow zones, though the magnitude of exceedances varies widely. According to hydrograph separation analysis, exceedances occur during both storm (runoff) and non-storm (baseflow) periods. These factors indicate that point sources are significant contributors to impairment in the Davis Creek watershed. However, it is possible that both point and non-point type sources contribute to exceedances of the E. coli standard in Davis Creek.

Results indicate the implementation strategy for the Davis Creek watershed will require BMPs targeting point sources (dominant under low flow/baseflow conditions). Table E-2 presents an allocation table of Load Duration Curve analysis statistics for Davis Creek E. coli and targeted implementation strategies for each source category covering the entire range of flow (Stiles, 2003). The implementation strategies listed in Table E-2 are a subset of the categories of BMPs and implementation strategies available for application to the Powell River watershed for reduction of E. coli loading and mitigation of water quality impairment from agricultural sources. Targeted implementation strategies and LDC analysis statistics for other impaired waterbodies and corresponding HUC-12 subwatersheds and drainage areas identified as predominantly agricultural source area types can be derived from the information and results available in Tables 11 and E-19.

Table E-19 presents LDC analyses (TMDLs, WLAs, LAs, and MOS) and PLRGs for all flow zones for all E. coli impaired waterbodies in the Powell River watershed.

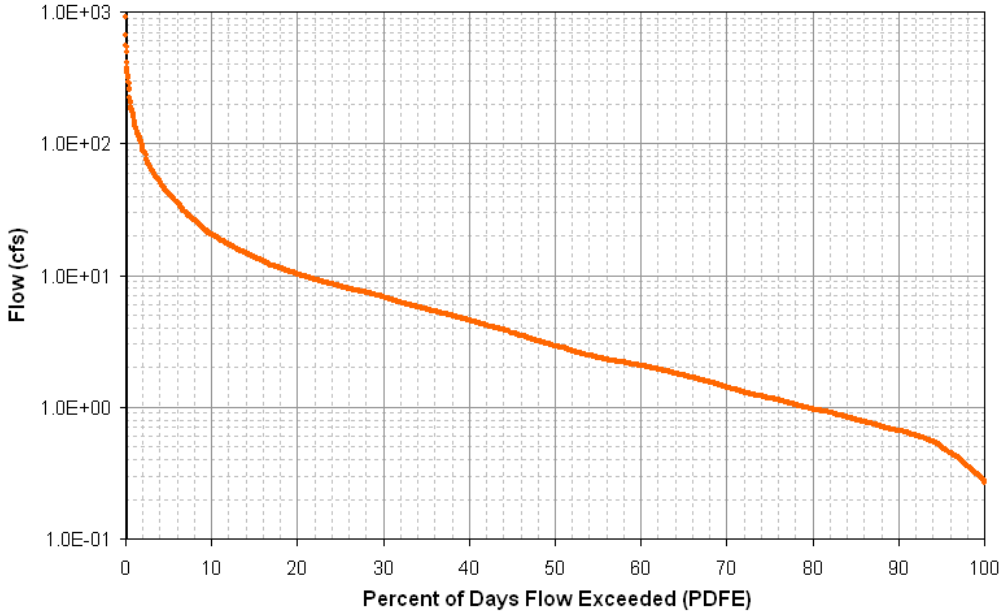


**Figure E-3. Flow Duration Curve for Davis Creek – RM14.6**

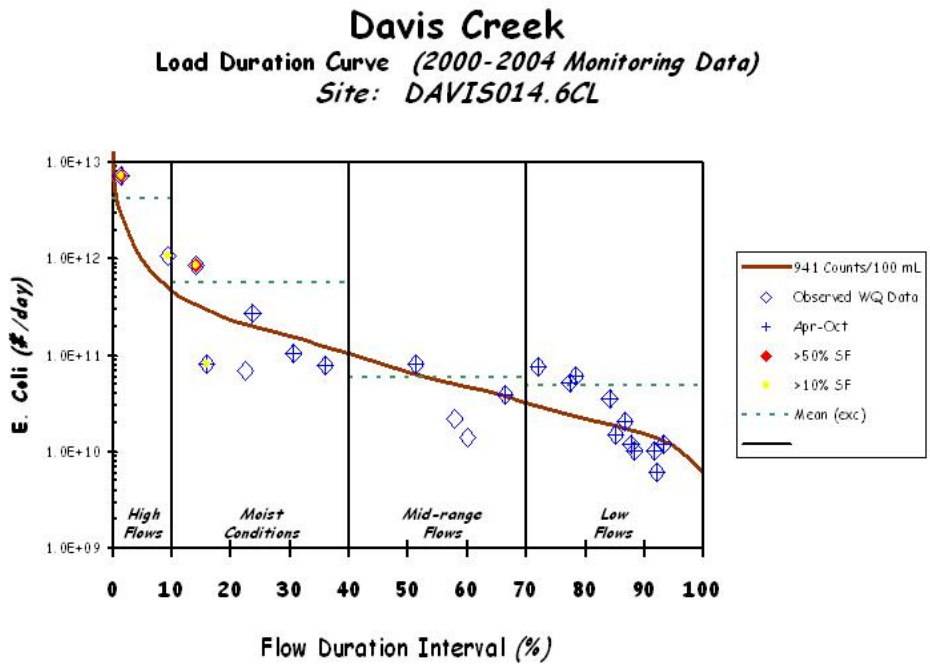


**Figure E-4. E. Coli Load Duration Curve for Davis Creek at Mile 14.6**

**Table E-2. Load Duration Curve Summary for Implementation Strategies (Example: Davis Creek subwatershed, HUC-12 060102060307) (4 Flow Zones).**

| Hydrologic Condition | | High | Moist | Mid-range* | Low* |
|---|---|---|---|---|---|
| % Time Flow Exceeded | | 0-10 | 10-40 | 40-70 | 70-100 |
| Davis Creek (060102060307) | Number of Samples | 2 | 6 | 4 | 11 |
| | % > 941 CFU/100 mL[1] | 100.0 | 33.3 | 50.0 | 45.5 |
| | Load Reduction[2] | 56.9 | 14.8 | 6.2 | 21.5 |
| TMDL (CFU/day) | | 9.646E+11 | 1.895E+11 | 5.474E+10 | 1.840E+10 |
| Margin of Safety (CFU/day) | | 1.646E+10 | 1.895E+10 | 5.474E+09 | 1.840E+09 |
| WLA (WWTFs) (CFU/day) | | NA | NA | NA | NA |
| WLA (MS4s) (CFU/day/acre)[3] | | NA | NA | NA | NA |
| LAs (CFU/day/acre)[3] | | 5.547E+07 | 1.090E+07 | 3.148E+06 | 1.058E+06 |
| Implementation Strategies[4] | | | | | |
| Pasture and Hayland Management | | H | H | M | L |
| Livestock Exclusion | | | | M | H |
| Fencing | | | | M | H |
| Manure Management | | H | H | M | L |
| Riparian Buffers | | L | M | H | M |
| Potential for source area contribution under given flow condition (H: High; M: Medium; L: Low) | | | | | |

* The low flow zone represents the critical conditions for E. coli loading in the Davis Creek subwatershed.
[1] Tennessee Maximum daily water quality criterion for E. coli.
[2] Reductions (percent) based on mean of observed percent load reductions in range.
[3] LAs and MS4s are expressed as daily load per unit area in order to provide for future changes in the distribution of LAs and MS4s (WLAs).
[4] Example Best Management Practices for Agricultural Source reduction. Actual BMPs applied may vary and should not be limited according to this grouping.

## E.3 Forestry Source Areas

There are no impaired waterbodies with corresponding HUC-12 subwatersheds or drainage areas classified as source area type predominantly forested, with the predominant source category being wildlife, in the Powell River watershed.

## E.4 Calculation of Percent Load Reduction Goals and Determination of Critical Flow Zones

In order to facilitate implementation, corresponding percent reductions in loading required to decrease existing, in-stream E. coli loads to TMDL target levels (percent load reduction goals) were calculated. The following example is from Davis Creek at mile 14.6.

1. For each flow zone, the mean of the percent exceedances of individual loads relative to their respective target maximum loads (at their respective PDFEs) was calculated. Each negative percent exceedance was assumed to be equal to zero.

| Date | Sample Conc. (CFU/100 mL) | Flow (cfs) | Existing Load (CFU/Day) | Target (TMDL) Load (CFU/Day) | Percent Reduction |
|---|---|---|---|---|---|
| 1/18/00 | 2419 | 14.66 | 8.68E+11 | 5.57E+13 | 61.1 |
| 7/18/00 | 261 | 12.84 | 8.20E+10 | 6.01E+12 | 0(-261) |
| 3/28/00 | 313 | 9.09 | 6.96E+10 | 7.21E+12 | 0(-201) |
| 4/24/00 | 1300 | 8.65 | 2.75E+11 | 2.99E+13 | 27.6 |
| 6/5/00 | 649 | 6.67 | 1.06E+11 | 1.49E+13 | 0(-45) |
| 8/9/00 | 613 | 5.30 | 7.95E+10 | 1.41E+13 | 0(-54) |
| Percent Load Reduction Goal (PLRG) for Moist Conditions (Mean) | | | | | **14.8** |

2. The PLRGs calculated for each of the flow zones, not including the high flow zone, were compared and the PLRG of the greatest magnitude indicates the critical flow zone for prioritizing implementation actions for Davis Creek at mile 14.6.

*Example –* *High Flow Zone Percent Load Reduction Goal = 56.9*
*Moist Conditions Flow Zone Percent Load Reduction Goal = 14.8*
*Mid-Range Flow Zone Percent Load Reduction Goal = 6.2*
***Low Flow Zone Percent Load Reduction Goal = 21.5***

Therefore, the critical flow zone for prioritization of Davis Creek implementation activities (at mile 14.6) is the Low Flow Zone and subsequently actions targeting point source controls.

PLRGs and critical flow zones of the other impaired waterbodies were derived in a similar manner and are shown in Table E-3.

**Table E-3. Summary of Critical Conditions for Impaired Waterbodies in the Powell River Watershed.**

| Waterbody ID | Moist | Mid-range | Dry | Low |
|---|---|---|---|---|
| Little Mulberry Creek | | ✓ | | |
| Mulberry Creek | | ✓ | | |
| Little Creek | | | | |
| Russell Creek | ✓ | | | |
| Gap Creek | | | | |
| Cawood Branch | | ✓ | | |
| Russell Branch | ✓ | | | |
| Davis Creek (1000) | | | | ✓ |
| Davis Creek (2000) | ✓ | | | |
| Davis Creek (3000) | ✓ | | | |
| Davis Creek (4000) | ✓ | | | |

* All Waterbody(ies) have 4 flow zones.

Geometric Mean Data

For cases where five or more samples were collected over a period of not more than 30 consecutive days, the geometric mean E. coli concentration was determined and compared to the target geometric mean E. coli concentration of 126 CFU/100 mL. If the sample geometric mean exceeded the target geometric mean concentration, the reduction required to reduce the sample geometric mean value to the target geometric mean concentration was calculated.

*Example:* *Monitoring Location = Russell Creek*
*Sampling Period = 6/22/04 – 7/14/04*
*Geometric Mean Concentration = 467.46 CFU/100 mL*
*Target Concentration = 126 CFU/100 mL*
*Reduction to Target = 73.1%*

For impaired waterbodies where monitoring data are limited to geometric mean data only, results can be utilized for general indication of relative impairment and, when plotted on a load duration curve, may indicate areas for prioritization of implementation efforts. For impaired waterbodies where both types of data are available, geometric mean data may be utilized to supplement the results of the individual flow zone calculations.

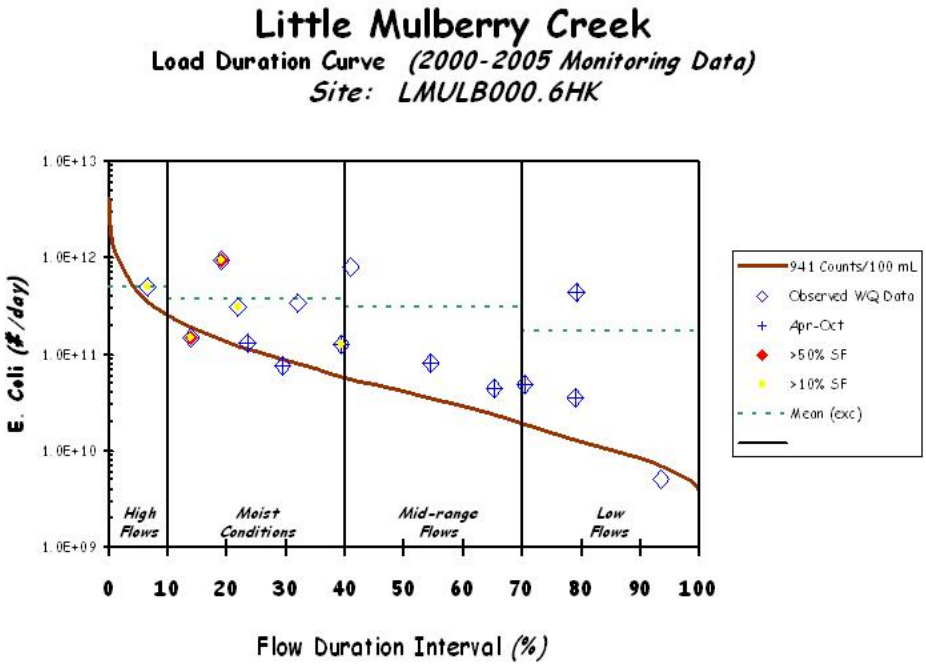


**Figure E-5. E. Coli Load Duration Curve for Little Mulberry Creek – RM0.6**

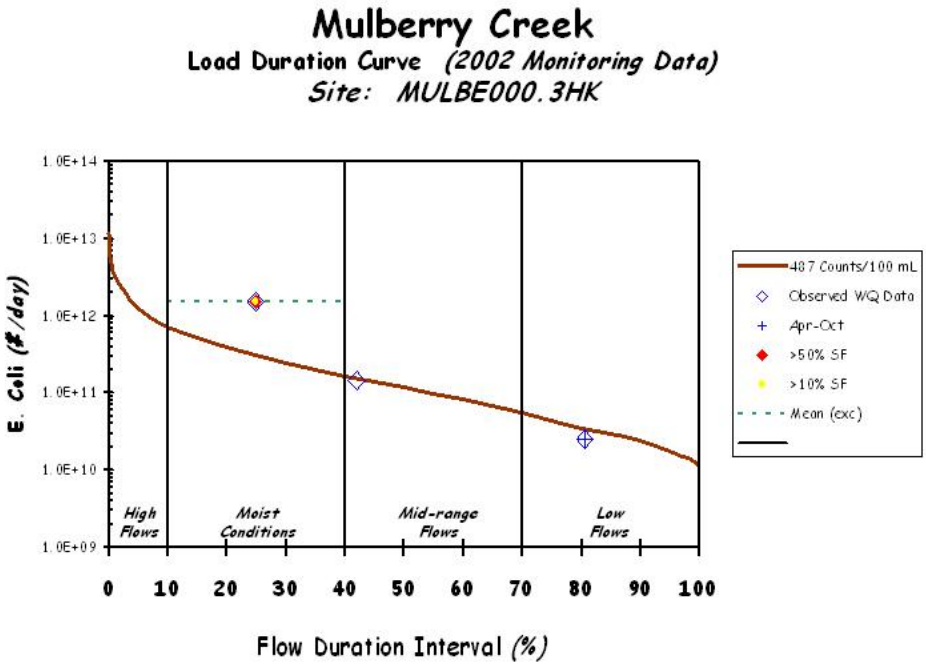


**Figure E-6. E. Coli Load Duration Curve for Mulberry Creek – RM0.3**

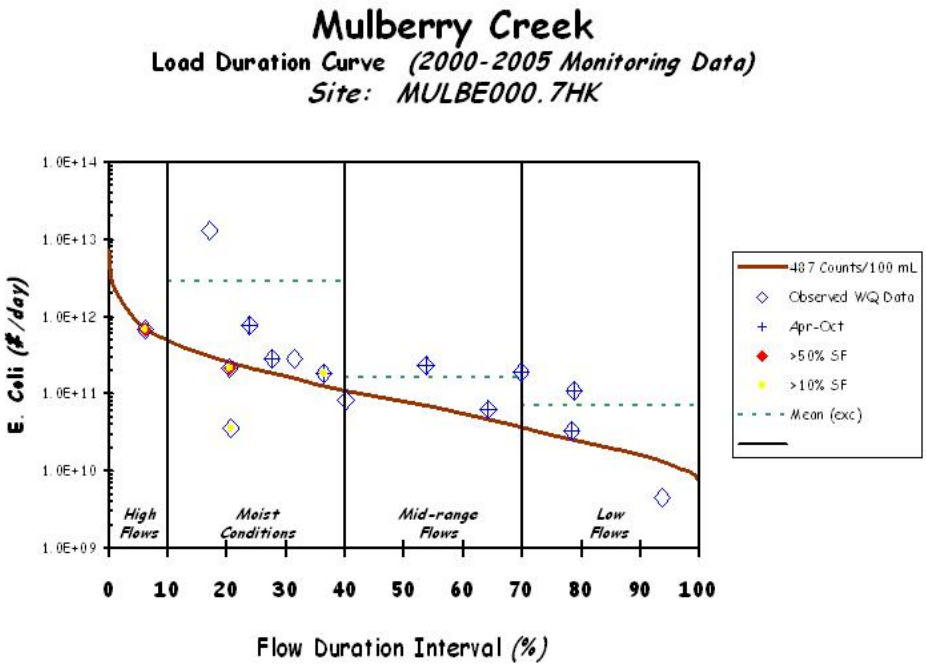


**Figure E-7. E. Coli Load Duration Curve for Mulberry Creek – RM0.7**

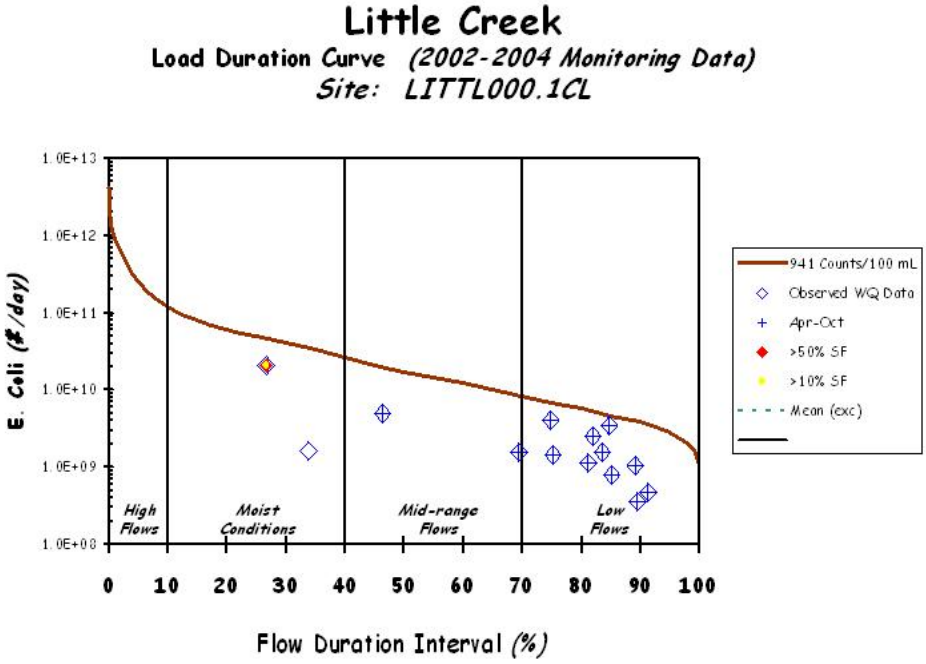


**Figure E-8. E. Coli Load Duration Curve for Little Creek**

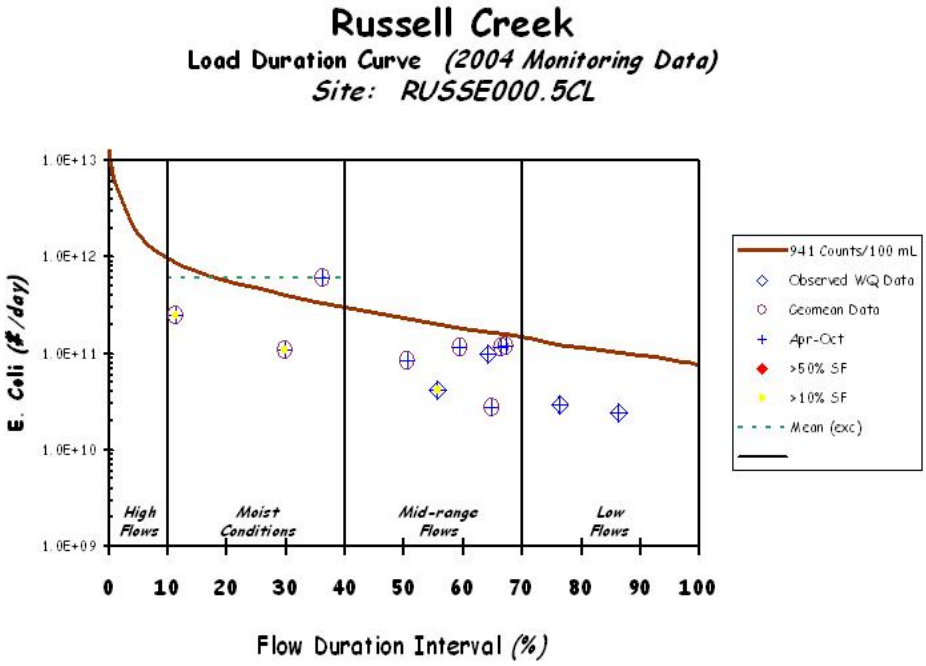


**Figure E-9. E. Coli Load Duration Curve for Russell Creek – RM0.5**

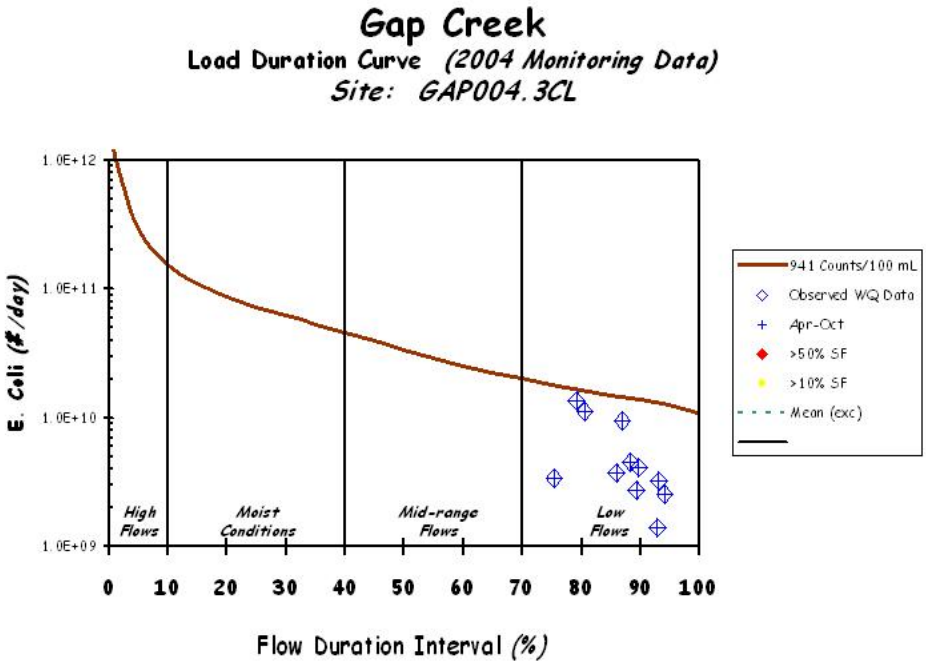


**Figure E-10. E. Coli Load Duration Curve for Gap Creek – RM4.3**

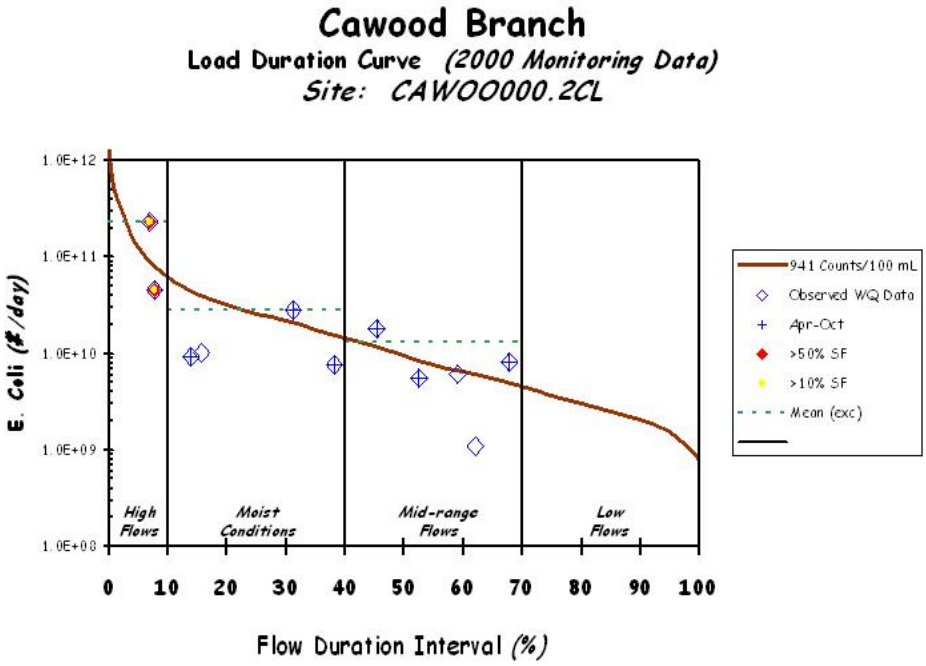


**Figure E-11. E. Coli Load Duration Curve for Cawood Branch**

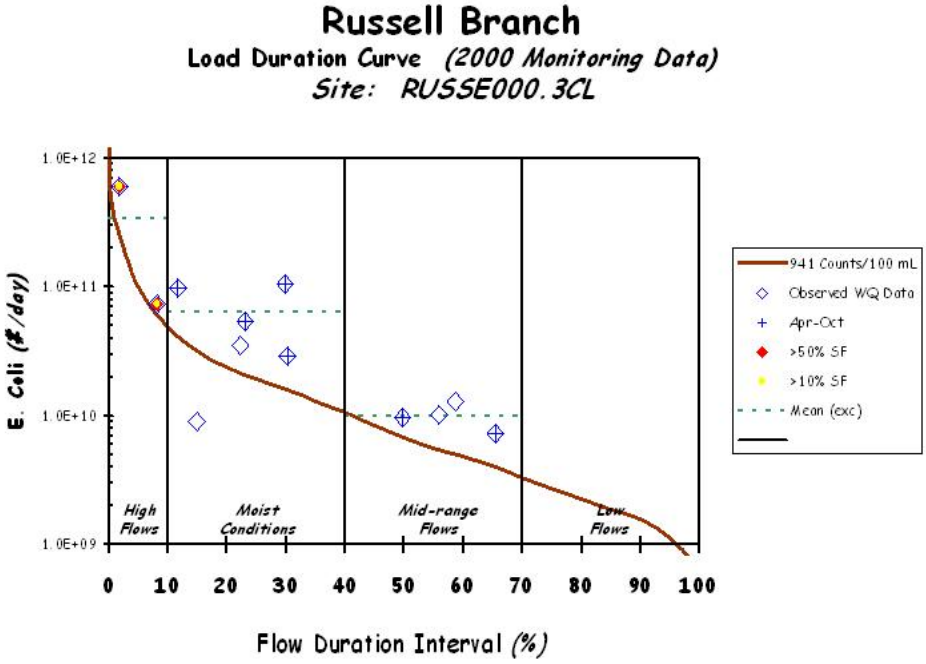


**Figure E-12. E. Coli Load Duration Curve for Russell Branch – RM0.3**

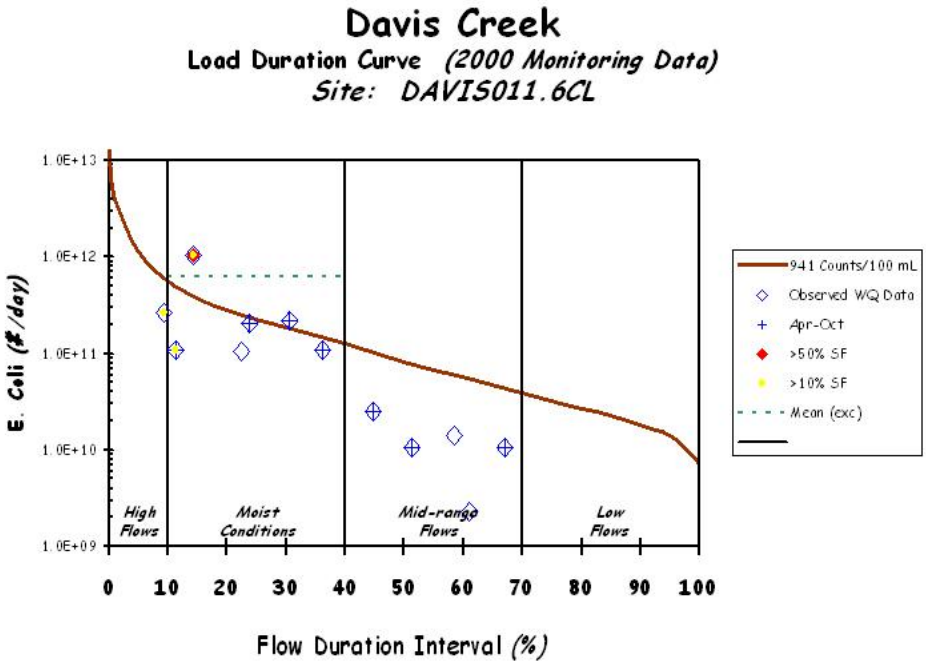


**Figure E-13. E. Coli Load Duration Curve for Davis Creek – RM11.6**

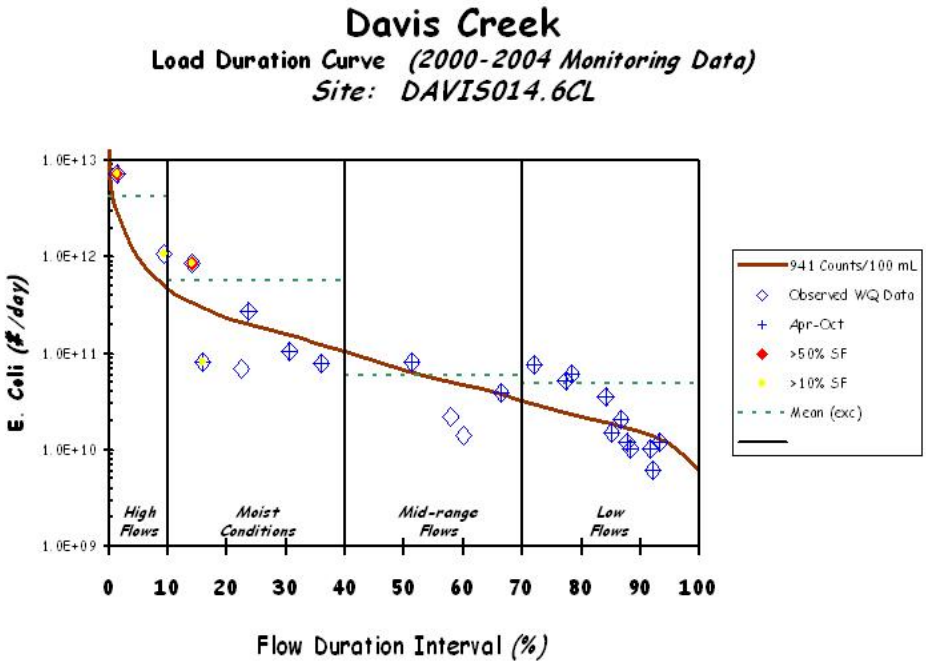


**Figure E-14. E. Coli Load Duration Curve for Davis Creek – RM14.6**

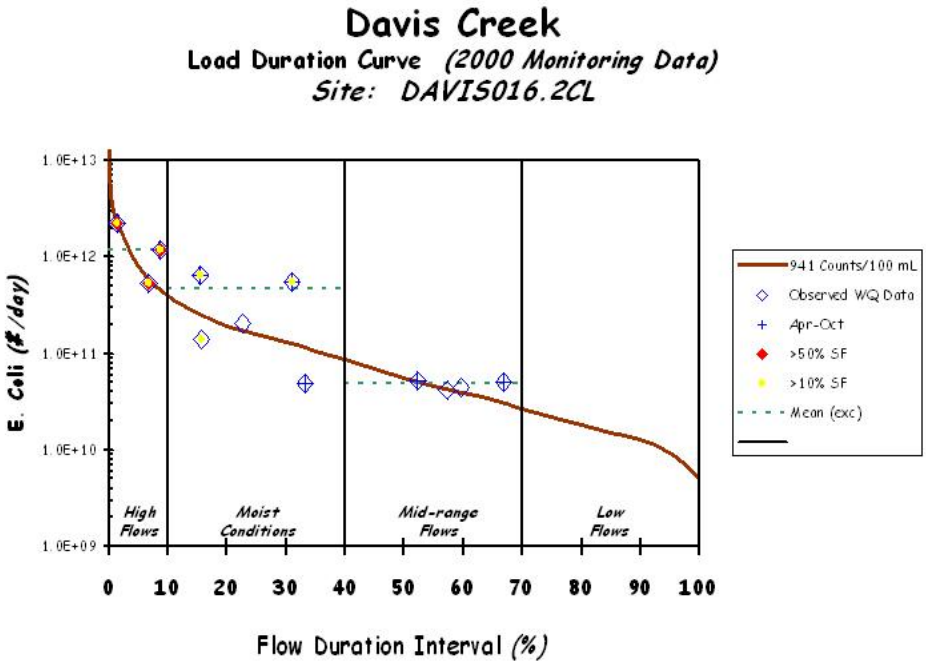


**Figure E-15. E. Coli Load Duration Curve for Davis Creek – RM16.2**

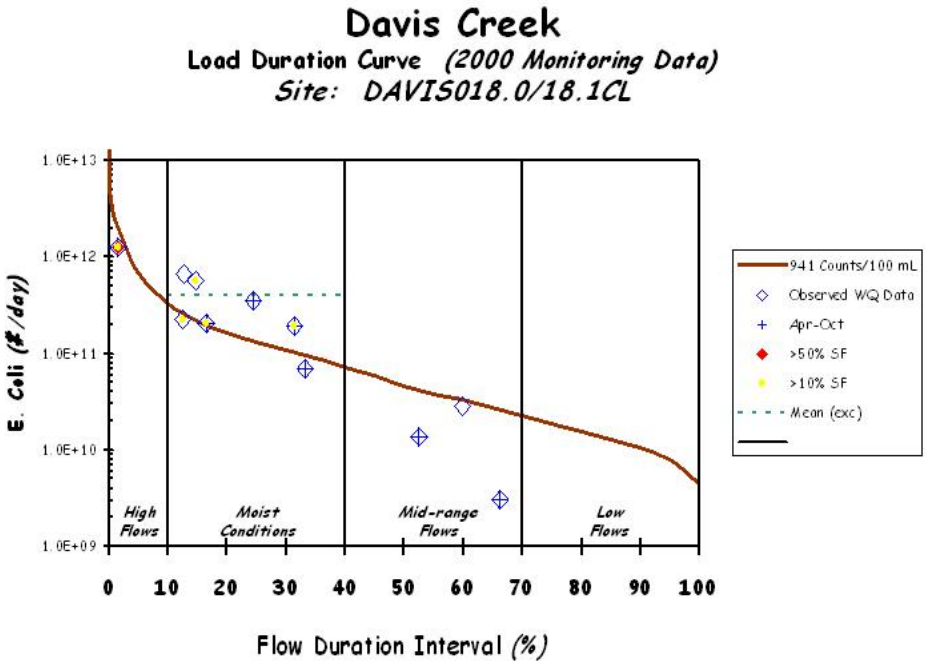


**Figure E-16. E. Coli Load Duration Curve for Davis Creek – RM18.0/18.1**

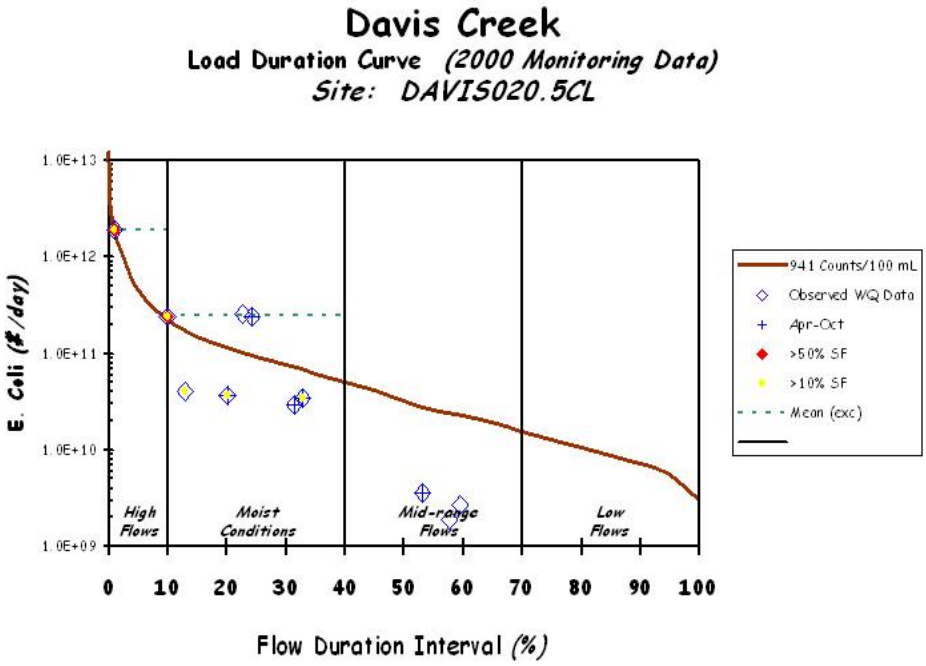


**Figure E-17. E. Coli Load Duration Curve for Davis Creek – RM20.5**

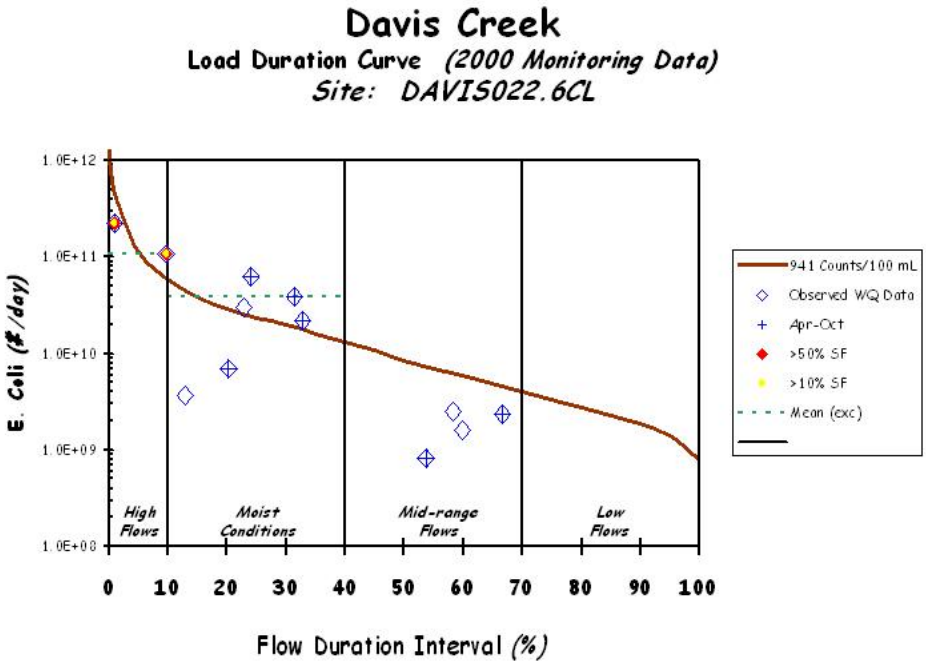


**Figure E-18. E. Coli Load Duration Curve for Davis Creek – RM22.6**

**Table E-4. Calculated Load Reduction Based on Daily Loading – Little Mulberry Creek – RM0.6**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 2/24/05 | High Flows | 15.66 | 6.6% | 1,310 | 5.02E+11 | **28.2** | **28.2** | **35.3** |
| 11/30/04 | Moist Conditions | 8.41 | 14.0% | 727 | 1.5E+11 | NR | | |
| 3/17/05 | | 6.13 | 19.2% | 6,380 | 9.56E+11 | **85.3** | | |
| 3/23/00 | | 5.29 | 22.0% | 2,419 | 3.13E+11 | **61.1** | | |
| 4/26/05 | | 4.90 | 23.5% | 1,100 | 1.32E+11 | **14.5** | | |
| 10/26/04 | | 3.82 | 29.5% | 816 | 7.63E+10 | NR | | |
| 12/21/04 | | 3.44 | 32.1% | 4,000 | 3.36E+11 | **76.5** | | |
| 7/29/04 | | 2.61 | 39.5% | 1,986 | 1.27E+11 | **52.6** | **41.4** | **44.4** |
| 1/25/05 | Mid-Range Flows | 2.41 | 41.1% | 13,540 | 7.99E+11 | **93.1** | | |
| 5/18/05 | | 1.52 | 54.6% | 2,160 | 8.05E+10 | **56.4** | | |
| 6/22/05 | | 1.04 | 65.4% | 1,730 | 4.39E+10 | **45.6** | **65.0** | **68.5** |
| 6/21/00 | Low Flows | 0.84 | 70.5% | 2,419 | 4.95E+10 | **61.1** | | |
| 9/14/04 | | 0.56 | 79.1% | 2,620 | 3.58E+10 | **64.1** | | |
| 8/25/04 | | 0.55 | 79.3% | 32,550 | 4.39E+11 | **97.1** | | |
| 12/6/00 | | 0.30 | 93.7% | 687 | 5.02E+09 | NR | **55.6** | **57.5** |

Note: NR = No reduction required
NA = Not applicable

**Table E-5. Calculated Load Reduction Based on Daily Loading – Mulberry Creek – RM0.3**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 11/6/02 | Moist Conditions | 25.77 | 25.0% | 2,419 | 1.52E+12 | **79.9** | **79.9** | **81.9** |
| 12/3/02 | Mid-Range Flows | 13.00 | 42.0% | 461 | 1.47E+11 | NR | **NR** | **NR** |
| 10/23/02 | Low Flows | 2.88 | 80.7% | 365 | 2.57E+10 | NR | **NR** | **NR** |

Note: NR = No reduction required
NA = Not applicable

**Table E-6. Calculated Load Reduction Based on Daily Loading – Mulberry Creek – RM0.7**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 2/24/05 | High Flows | 60.26 | 6.2% | 461 | 6.8E+11 | NR | **NR** | **5.0** |
| 11/30/04 | Moist Conditions | 25.91 | 17.1% | 20,460 | 1.3E+13 | **97.6** | | |
| 3/17/05 | | 21.60 | 20.5% | 410 | 2.17E+11 | NR | | |
| 3/23/00 | | 21.26 | 20.8% | 68 | 3.54E+10 | NR | | |
| 4/26/05 | | 18.21 | 24.0% | 1,733 | 7.72E+11 | **71.9** | | |
| 10/26/04 | | 15.39 | 27.7% | 770 | 2.9E+11 | **36.8** | | |
| 12/21/04 | | 13.25 | 31.4% | 866 | 2.81E+11 | **43.8** | | |
| 7/29/04 | | 10.82 | 36.5% | 687 | 1.82E+11 | **29.1** | **34.9** | **38.3** |
| 1/25/05 | Mid-Range Flows | 9.35 | 40.3% | 365 | 8.35E+10 | NR | | |
| 5/18/05 | | 5.93 | 53.7% | 1,613 | 2.34E+11 | **69.8** | | |
| 6/22/05 | | 4.02 | 64.4% | 630 | 6.19E+10 | **22.7** | | |
| 6/21/00 | | 3.21 | 69.9% | 2,419 | 1.9E+11 | **79.9** | **43.1** | **46.3** |
| 9/14/04 | Low Flows | 2.13 | 78.5% | 649 | 3.38E+10 | **25.0** | | |
| 8/25/04 | | 2.10 | 78.9% | 2,110 | 1.08E+11 | **76.9** | | |
| 12/6/00 | | 1.09 | 93.9% | 172 | 4.61E+09 | NR | **34.0** | **37.3** |

Note: NR = No reduction required
NA = Not applicable

**Table E-7. Calculated Load Reduction Based on Daily Loading – Little Creek**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 11/5/02 | Moist Conditions | 2.07 | 26.7% | 416 | 2.11E+10 | NR | | |
| 12/3/02 | | 1.52 | 33.9% | 43 | 1.6E+09 | NR | **NR** | **NR** |
| 10/23/02 | Mid-Range Flows | 0.86 | 46.5% | 238 | 5.02E+09 | NR | | |
| 7/8/04 | | 0.37 | 69.5% | 173 | 1.56E+09 | NR | **NR** | **NR** |
| 8/2/04 | Low Flows | 0.30 | 75.0% | 548 | 3.99E+09 | NR | | |
| 7/19/04 | | 0.29 | 75.3% | 201 | 1.45E+09 | NR | | |
| 8/17/04 | | 0.24 | 81.2% | 197 | 1.14E+09 | NR | | |
| 8/19/04 | | 0.23 | 82.1% | 461 | 2.55E+09 | NR | | |
| 8/26/04 | | 0.21 | 83.7% | 299 | 1.57E+09 | NR | | |
| 9/21/04 | | 0.20 | 84.8% | 687 | 3.42E+09 | NR | | |
| 9/2/04 | | 0.20 | 85.2% | 161 | 7.83E+08 | NR | | |
| 10/4/04 | | 0.17 | 89.3% | 249 | 1.06E+09 | NR | | |
| 10/6/04 | | 0.17 | 89.7% | 86 | 3.61E+08 | NR | | |
| 10/11/04 | | 0.16 | 91.4% | 121 | 4.75E+08 | NR | **NR** | **NR** |
| **Geomean of All Samples[a]** | | | | **220** | | **42.7** | | |

Note: NR = No reduction required
NA = Not applicable

[a] Analysis of monitoring data by traditional methods suggests no load reduction is necessary. However, TDEC believes there is reasonable potential for impairment. Therefore, a load reduction was calculated based on the geomean of all sampling data and the geomean criteria (126).

**Table E-8. Calculated Load Reduction Based on Daily Loading – Russell Creek – RM0.5**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 6/28/04 | Moist Conditions | 39.14 | 11.4% | 260 | 2.49E+11 | NR | | |
| 7/1/04 | | 17.58 | 29.9% | 248 | 1.07E+11 | NR | | |
| 6/22/04 | | 14.39 | 36.2% | 1,733 | 6.1E+11 | **45.7** | **15.2** | **17.0** |
| 7/7/04 | Mid-Range Flows | 9.99 | 50.6% | 345 | 8.44E+10 | NR | | |
| 9/23/04 | | 8.84 | 55.7% | 192 | 4.15E+10 | NR | | |
| 7/14/04 | | 8.01 | 59.6% | 579 | 1.14E+11 | NR | | |
| 9/29/04 | | 7.24 | 64.3% | 548 | 9.71E+10 | NR | | |
| 7/20/04 | | 7.11 | 65.0% | 157 | 2.73E+10 | NR | | |
| 8/3/04 | | 6.85 | 66.6% | 687 | 1.15E+11 | NR | | |
| 7/22/04 | | 6.75 | 67.3% | 727 | 1.2E+11 | NR | **NR** | **NR** |
| 8/16/04 | Low Flows | 5.43 | 76.5% | 219 | 2.91E+10 | NR | | |
| 9/2/04 | | 4.44 | 86.4% | 225 | 2.44E+10 | NR | **NR** | **NR** |

Note: NR = No reduction required
NA = Not applicable

**Table E-9. Calculated Load Reduction Based on Geomean Data – Russell Creek – RM0.5**

| Sample Date | Flow | PDFE | Concentration | Geometric Mean | Calculated Reduction | |
|---|---|---|---|---|---|---|
| | | | | | to Target GM (126 CFU/100 ml) | to Target – MOS (113 CFU/100 ml) |
| | [cfs] | [%] | [CFU/100 ml] | [CFU/100 ml] | [%] | [%] |
| 6/22/04 | 14.39 | 36.2% | 1,733 | | | |
| 6/28/04 | 39.14 | 11.4% | 260 | | | |
| 7/1/04 | 17.58 | 29.9% | 248 | | | |
| 7/7/04 | 9.99 | 50.6% | 345 | | | |
| 7/14/04 | 8.01 | 59.6% | 579 | 467.46 | **73.1** | **75.8** |
| 7/20/04 | 7.11 | 65.0% | 157 | 389.74 | 67.7 | 71.0 |
| 7/22/04 | 6.75 | 67.3% | 727 | | | |
| 8/3/04 | 6.85 | 66.6% | 687 | 435.49 | 71.2 | 74.1 |

Note: Geometric Mean is calculated whenever 5 or more samples are collected over a period of not more than 30 consecutive days.

**Table E-10. Calculated Load Reduction Based on Daily Loading – Gap Creek**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 7/8/04 | Low Flows | 0.77 | 75.5% | 179 | 3.37E+09 | NR | **NR** | **NR** |
| 8/2/04 | | 0.72 | 79.3% | 770 | 1.35E+10 | NR | | |
| 7/19/04 | | 0.70 | 80.6% | 649 | 1.11E+10 | NR | | |
| 8/17/04 | | 0.64 | 86.1% | 238 | 3.73E+09 | NR | | |
| 8/19/04 | | 0.63 | 87.0% | 613 | 9.45E+09 | NR | | |
| 8/26/04 | | 0.62 | 88.4% | 299 | 4.52E+09 | NR | | |
| 9/21/04 | | 0.61 | 89.5% | 186 | 2.76E+09 | NR | | |
| 9/2/04 | | 0.60 | 89.9% | 276 | 4.06E+09 | NR | | |
| 10/4/04 | | 0.58 | 93.0% | 99 | 1.4E+09 | NR | | |
| 10/6/04 | | 0.57 | 93.3% | 228 | 3.2E+09 | NR | | |
| 10/11/04 | | 0.56 | 94.2% | 186 | 2.56E+09 | NR | | |
| **Geomean of All Samples[a]** | | | | **280** | | **55.0** | | |

Note: NR = No reduction required
NA = Not applicable

[a] Analysis of monitoring data by traditional methods suggests no load reduction is necessary. However, TDEC believes there is reasonable potential for impairment. Therefore, a load reduction was calculated based on the geomean of all sampling data and the geomean criteria (126).

**Table E-11. Calculated Load Reduction Based on Daily Loading – Cawood Branch**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 1/20/00 | High Flows | 3.96 | 6.9% | 2,419 | 2.34E+11 | **61.1** | | |
| 4/26/00 | | 3.56 | 7.7% | 517 | 4.5E+10 | NR | **30.5** | **32.5** |
| 7/17/00 | Moist Conditions | 1.96 | 14.0% | 196 | 9.38E+09 | NR | | |
| 2/24/00 | | 1.74 | 15.7% | 236 | 1.01E+10 | NR | | |
| 6/6/00 | | 0.89 | 31.3% | 1,300 | 2.84E+10 | **27.6** | | |
| 8/9/00 | | 0.68 | 38.3% | 461 | 7.62E+09 | NR | **6.9** | **8.7** |
| 5/22/00 | Mid-Range Flows | 0.52 | 45.4% | 1,414 | 1.8E+10 | **33.5** | | |
| 9/26/00 | | 0.37 | 52.5% | 613 | 5.62E+09 | NR | | |
| 11/14/00 | | 0.29 | 59.2% | 866 | 6.16E+09 | NR | | |
| 12/11/00 | | 0.26 | 62.1% | 166 | 1.08E+09 | NR | | |
| 10/25/00 | | 0.22 | 67.9% | 1,553 | 8.18E+09 | **39.4** | **14.6** | **17.6** |

Note: NR = No reduction required
NA = Not applicable

**Table E-12. Calculated Load Reduction Based on Daily Loading – Russell Branch – RM0.3**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 5/23/00 | High Flows | 10.21 | 1.8% | 2,419 | 6.04E+11 | **61.1** | | |
| 1/20/00 | | 2.68 | 8.1% | 1,120 | 7.34E+10 | **16.0** | **38.5** | **44.7** |
| 7/18/00 | Moist Conditions | 1.78 | 11.8% | 2,280 | 9.96E+10 | **58.7** | | |
| 2/24/00 | | 1.40 | 15.0% | 260 | 8.93E+09 | NR | | |
| 3/28/00 | | 0.93 | 22.4% | 1,553 | 3.55E+10 | **39.4** | | |
| 4/25/00 | | 0.91 | 23.1% | 2,419 | 5.37E+10 | **61.1** | | |
| 8/8/00 | | 0.70 | 29.9% | 6,086 | 1.05E+11 | **84.5** | | |
| 6/6/00 | | 0.69 | 30.2% | 1,733 | 2.91E+10 | **45.7** | **48.2** | **51.8** |
| 9/26/00 | Mid-Range Flows | 0.29 | 49.9% | 1,350 | 9.74E+09 | **30.3** | | |
| 11/15/00 | | 0.24 | 55.9% | 1,733 | 1.01E+10 | **45.7** | | |
| 12/12/00 | | 0.22 | 58.8% | 2,419 | 1.29E+10 | **61.1** | | |
| 10/25/00 | | 0.17 | 65.6% | 1,733 | 7.33E+09 | **45.7** | **45.7** | **51.1** |

Note: NR = No reduction required
NA = Not applicable

**Table E-13. Calculated Load Reduction Based on Daily Loading – Davis Creek – RM11.6**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 2/22/00 | High Flows | 26.54 | 9.3% | 411 | 2.67E+11 | NR | **NR** | **NR** |
| 7/17/00 | Moist Conditions | 21.42 | 11.4% | 205 | 1.07E+11 | NR | | |
| 1/18/00 | | 17.35 | 14.3% | 2,419 | 1.03E+12 | 61.1 | | |
| 3/28/00 | | 10.93 | 22.6% | 387 | 1.04E+11 | NR | | |
| 4/24/00 | | 10.37 | 23.9% | 816 | 2.07E+11 | NR | | |
| 6/5/00 | | 7.99 | 30.6% | 1,120 | 2.19E+11 | 16.0 | | |
| 8/9/00 | | 6.38 | 36.2% | 687 | 1.07E+11 | NR | **12.8** | **14.9** |
| 5/22/00 | Mid-Range Flows | 4.49 | 44.8% | 228 | 2.5E+10 | NR | | |
| 9/26/00 | | 3.34 | 51.4% | 131 | 1.07E+10 | NR | | |
| 11/14/00 | | 2.60 | 58.5% | 225 | 1.43E+10 | NR | | |
| 12/11/00 | | 2.41 | 61.2% | 39 | 2.3E+09 | NR | | |
| 10/25/00 | | 1.92 | 67.2% | 225 | 1.06E+10 | NR | **NR** | **NR** |

Note: NR = No reduction required
NA = Not applicable

**Table E-14. Calculated Load Reduction Based on Daily Loading – Davis Creek – RM14.6**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 5/24/00 | High Flows | 122.86 | 1.4% | 2,419 | 7.27E+12 | **61.1** | | |
| 2/22/00 | | 22.06 | 9.3% | 1,986 | 1.07E+12 | **52.6** | **56.9** | **61.2** |
| 1/18/00 | Moist Conditions | 14.66 | 14.1% | 2,419 | 8.68E+11 | **61.1** | | |
| 7/18/00 | | 12.84 | 16.0% | 261 | 8.20E+10 | NR | | |
| 3/28/00 | | 9.09 | 22.6% | 313 | 6.96E+10 | NR | | |
| 4/24/00 | | 8.65 | 23.8% | 1,300 | 2.75E+11 | **27.6** | | |
| 6/5/00 | | 6.67 | 30.6% | 649 | 1.06E+11 | NR | | |
| 8/9/00 | | 5.30 | 36.1% | 613 | 7.95E+10 | NR | **14.8** | **16.6** |
| 9/26/00 | Mid-Range Flows | 2.76 | 51.4% | 1,190 | 8.04E+10 | **20.9** | | |
| 11/14/00 | | 2.19 | 57.9% | 411 | 2.20E+10 | NR | | |
| 12/12/00 | | 2.05 | 60.2% | 285 | 1.43E+10 | NR | | |
| 10/24/00 | | 1.65 | 66.5% | 980 | 3.95E+10 | **4.0** | **6.2** | **10.6** |
| 7/8/04 | Low Flows | 1.29 | 72.1% | 2,419 | 7.63E+10 | **61.1** | | |
| 8/2/04 | | 1.07 | 77.4% | 1,986 | 5.18E+10 | **52.6** | | |
| 7/19/04 | | 1.03 | 78.4% | 2,419 | 6.08E+10 | **61.1** | | |
| 8/17/04 | | 0.83 | 84.2% | 1,733 | 3.54E+10 | **45.7** | | |
| 8/19/04 | | 0.80 | 85.2% | 770 | 1.50E+10 | NR | | |
| 8/26/04 | | 0.75 | 86.8% | 1,120 | 2.06E+10 | **16.0** | | |
| 9/21/04 | | 0.72 | 87.8% | 687 | 1.21E+10 | NR | | |
| 9/2/04 | | 0.70 | 88.5% | 613 | 1.04E+10 | NR | | |
| 10/4/04 | | 0.62 | 91.8% | 687 | 1.04E+10 | NR | | |
| 10/6/04 | | 0.61 | 92.2% | 411 | 6.09E+09 | NR | | |
| 10/11/04 | | 0.57 | 93.5% | 866 | 1.20E+10 | NR | **21.5** | **24.1** |

Note: NR = No reduction required
NA = Not applicable

**Table E-15. Calculated Load Reduction Based on Daily Loading – Davis Creek – RM16.2**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 5/24/00 | High Flows | 105.63 | 1.3% | 866 | 2.24E+12 | NR | | |
| 1/20/00 | | 25.78 | 6.7% | 846 | 5.34E+11 | NR | | |
| 4/26/00 | | 20.02 | 8.7% | 2,419 | 1.19E+12 | **61.1** | **20.4** | **22.4** |
| 7/18/00 | Moist Conditions | 10.99 | 15.6% | 2,400 | 6.45E+11 | **60.8** | | |
| 2/24/00 | | 10.97 | 15.7% | 517 | 1.39E+11 | NR | | |
| 3/28/00 | | 7.49 | 22.7% | 1,120 | 2.05E+11 | **16.0** | | |
| 8/8/00 | | 5.40 | 31.1% | 4,135 | 5.46E+11 | **77.2** | | |
| 6/7/00 | | 4.89 | 33.2% | 411 | 4.91E+10 | NR | **30.8** | **33.7** |
| 9/27/00 | Mid-Range Flows | 2.17 | 52.3% | 970 | 5.15E+10 | **3.0** | | |
| 11/14/00 | | 1.82 | 57.4% | 921 | 4.1E+10 | NR | | |
| 12/12/00 | | 1.71 | 59.7% | 1,046 | 4.37E+10 | **10.0** | | |
| 10/25/00 | | 1.33 | 66.9% | 1,560 | 5.08E+10 | **39.7** | **13.2** | **21.4** |

Note: NR = No reduction required
NA = Not applicable

**Table E-16. Calculated Load Reduction Based on Daily Loading – Davis Creek – RM18.0/18.1**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 5/23/00 | High Flows | 85.47 | 1.5% | 613 | 3.55E+10 | NR | **NR** | **NR** |
| 2/23/00 | Moist Conditions | 11.45 | 12.6% | 816 | 7.34E+10 | NR | | |
| 1/19/00 | | 11.27 | 12.8% | 2,419 | 6.04E+11 | **61.1** | | |
| 3/23/00 | | 9.71 | 14.8% | 2,419 | 9.96E+10 | **61.1** | | |
| 7/18/00 | | 8.66 | 16.5% | 980 | 1.05E+11 | **4.0** | | |
| 4/25/00 | | 5.88 | 24.5% | 2,419 | 8.93E+09 | **61.1** | | |
| 8/8/00 | | 4.44 | 31.5% | 1,749 | 2.91E+10 | **46.2** | | |
| 6/7/00 | | 4.10 | 33.3% | 687 | 5.37E+10 | NR | **33.4** | **37.2** |
| 9/27/00 | Mid-Range Flows | 1.81 | 52.6% | 310 | 9.74E+09 | NR | | |
| 12/12/00 | | 1.43 | 60.0% | 816 | 1.29E+10 | NR | | |
| 10/24/00 | | 1.14 | 66.3% | 108 | 1.01E+10 | NR | **NR** | **NR** |

Note: NR = No reduction required
NA = Not applicable

**Table E-17. Calculated Load Reduction Based on Daily Loading – Davis Creek – RM20.5**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 5/23/00 | High Flows | 78.52 | 1.0% | 980 | 3.55E+10 | **4.0** | **4.0** | **13.6** |
| 1/19/00 | Moist Conditions | 9.90 | 10.0% | 980 | 6.04E+11 | **4.0** | | |
| 2/23/00 | | 7.62 | 13.0% | 219 | 7.34E+10 | NR | | |
| 7/18/00 | | 4.93 | 20.2% | 300 | 1.05E+11 | NR | | |
| 3/28/00 | | 4.34 | 22.8% | 2,419 | 9.96E+10 | **61.1** | | |
| 4/25/00 | | 4.12 | 24.3% | 2,419 | 8.93E+09 | **61.1** | | |
| 6/6/00 | | 3.13 | 31.4% | 387 | 5.37E+10 | NR | | |
| 8/8/00 | | 2.92 | 32.9% | 488 | 2.91E+10 | NR | **16.3** | **19.6** |
| 9/27/00 | Mid-Range Flows | 1.23 | 53.1% | 118 | 9.74E+09 | NR | | |
| 11/15/00 | | 1.05 | 57.8% | 73 | 1.29E+10 | NR | | |
| 12/12/00 | | 0.99 | 59.5% | 111 | 7.33E+09 | NR | **NR** | **NR** |

Note: NR = No reduction required
NA = Not applicable

**Table E-18. Calculated Load Reduction Based on Daily Loading – Davis Creek – RM22.6**

| Sample Date | Flow Regime | Flow | PDFE | Concentration | Load | % Reduction to Achieve TMDL | Average of Load Reductions | % Reduction to TMDL – MOS |
|---|---|---|---|---|---|---|---|---|
| | | [cfs] | [%] | [CFU/100 ml] | [CFU/day] | [%] | [%] | [%] |
| 5/23/00 | High Flows | 20.08 | 1.0% | 461 | 2.27E+11 | NR | | |
| 1/19/00 | | 2.56 | 9.8% | 1,733 | 1.09E+11 | **45.7** | **22.9** | **25.6** |
| 2/23/00 | Moist Conditions | 1.95 | 13.0% | 77 | 3.67E+09 | NR | | |
| 7/18/00 | | 1.26 | 20.3% | 222 | 6.83E+09 | NR | | |
| 3/28/00 | | 1.11 | 22.9% | 1,120 | 3.04E+10 | **16.0** | | |
| 4/25/00 | | 1.06 | 24.2% | 2,419 | 6.27E+10 | **61.1** | | |
| 6/6/00 | | 0.80 | 31.5% | 1,986 | 3.88E+10 | **52.6** | | |
| 8/8/00 | | 0.75 | 32.9% | 1,200 | 2.2E+10 | **21.6** | **25.5** | **29.4** |
| 9/27/00 | Mid-Range Flows | 0.31 | 53.7% | 108 | 8.32E+08 | NR | | |
| 11/15/00 | | 0.27 | 58.3% | 378 | 2.48E+09 | NR | | |
| 12/12/00 | | 0.25 | 60.0% | 260 | 1.62E+09 | NR | | |
| 10/24/00 | | 0.20 | 66.8% | 479 | 2.34E+09 | NR | **NR** | **NR** |

Note: NR = No reduction required
NA = Not applicable

**Table E-19 Summary of TMDLs, WLAs, & LAs expressed as daily loads for Impaired Waterbodies in the Powell River Watershed (HUC 06010206)**

| Waterbody Description (TN06010206__) | Hydrologic Condition | | | Flow[a] | PLRG | TMDL | MOS | WLAs | | | | LAs |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | Flow Regime | PDFE Range | Flow Range | | | | | WWTFs [b] | CS | CAFOs | MS4s | |
| | | [%] | [cfs] | [cfs] | [%] | [CFU/d] | [CFU/d] | [CFU/d] | | | [CFU/d/ac] | [CFU/d/ac] |
| **Little Mulberry Creek** Waterbody ID: **007 – 0710** HUC-12: 0205 | High Flows | 0 – 10 | 11.09 – 153.5 | 18.86 | 28.2 | $4.338 \times 10^{11}$ | $4.338 \times 10^{10}$ | NA | NA | NA | NA | $1.486 \times 10^{8}$ |
| | Moist | 10 – 40 | 2.52 – 11.09 | 4.69 | 41.4 | $1.079 \times 10^{11}$ | $1.079 \times 10^{10}$ | | | | | $3.694 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 0.85 – 2.52 | 1.50 | 65.0 | $3.450 \times 10^{10}$ | $3.450 \times 10^{9}$ | | | | | $1.182 \times 10^{7}$ |
| | Low Flows | 70 – 100 | 0.17 – 0.85 | 0.44 | 55.6 | $1.012 \times 10^{10}$ | $1.012 \times 10^{9}$ | | | | | $3.466 \times 10^{6}$ |
| **Mulberry Creek** Waterbody ID: **007 – 0700** HUC-12: 0205 | High Flows | 0 – 10 | 41.63 – 495.8 | 71.28 | NR | $8.554 \times 10^{11}$ | $8.554 \times 10^{10}$ | NA | NA | NA | NA | $7.827 \times 10^{7}$ |
| | Moist | 10 – 40 | 9.41 – 41.63 | 17.58 | 34.9 | $2.110 \times 10^{11}$ | $2.110 \times 10^{10}$ | | | | | $1.930 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 3.18 – 9.41 | 5.66 | 43.1 | $6.792 \times 10^{10}$ | $6.792 \times 10^{9}$ | | | | | $6.215 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.65 – 3.18 | 1.67 | 34.0 | $2.004 \times 10^{10}$ | $2.004 \times 10^{9}$ | | | | | $1.834 \times 10^{6}$ |
| **Little Creek** Waterbody ID: **007 – 0100** HUC-12: 0301 | High Flows | 0 – 10 | 28.15 – 536.7 | 54.71 | 42.7 | $1.258 \times 10^{11}$ | $1.258 \times 10^{10}$ | NA | NA | NA | NA | $2.487 \times 10^{8}$ |
| | Moist | 10 – 40 | 5.96 – 28.15 | 10.99 | | $2.528 \times 10^{11}$ | $2.528 \times 10^{10}$ | | | | | $4.995 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 1.95 – 5.96 | 3.20 | | $7.360 \times 10^{10}$ | $7.360 \times 10^{9}$ | | | | | $1.455 \times 10^{7}$ |
| | Low Flows | 70 – 100 | 0.35 – 1.95 | 1.11 | | $2.553 \times 10^{10}$ | $2.553 \times 10^{9}$ | | | | | $5.045 \times 10^{6}$ |
| **Russell Creek** Waterbody ID: **008 – 1000** HUC-12: 0302 | High Flows | 0 – 10 | 43.11 – 681.2 | 75.66 | 73.1 | $1.740 \times 10^{12}$ | $1.740 \times 10^{11}$ | $2.315 \times 10^{9}$ | 0 | NA | NA | $1.426 \times 10^{8}$ |
| | Moist | 10 – 40 | 12.97 – 43.11 | 20.84 | | $4.793 \times 10^{11}$ | $4.793 \times 10^{10}$ | | | | | $3.773 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 6.39 – 12.97 | 9.02 | | $2.075 \times 10^{11}$ | $2.075 \times 10^{10}$ | | | | | $1.512 \times 10^{7}$ |
| | Low Flows | 70 – 100 | 3.28 – 6.39 | 4.54 | | $1.044 \times 10^{11}$ | $1.044 \times 10^{10}$ | | | | | $6.547 \times 10^{6}$ |
| **Gap Creek** Waterbody ID: **006 – 0200** HUC-12: 0304 | High Flows | 0 – 10 | 6.71 – 159.0 | 12.94 | 55.0 | $2.976 \times 10^{11}$ | $2.976 \times 10^{10}$ | $1.843 \times 10^{9}$ | 0 | NA | NA | $5.587 \times 10^{7}$ |
| | Moist | 10 – 40 | 1.98 – 6.71 | 3.15 | | $7.245 \times 10^{10}$ | $7.245 \times 10^{9}$ | | | | | $1.331 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 0.88 – 1.98 | 1.25 | | $2.875 \times 10^{10}$ | $2.875 \times 10^{9}$ | | | | | $5.048 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.48 – 0.88 | 0.65 | | $1.495 \times 10^{10}$ | $1.495 \times 10^{9}$ | | | | | $2.439 \times 10^{6}$ |
| **Cawood Branch** Waterbody ID: **026 – 0100** HUC-12: 0307 | High Flows | 0 – 10 | 2.69 – 67.1 | 5.41 | 30.5 | $1.244 \times 10^{11}$ | $31.244 \times 10^{10}$ | NA | NA | NA | NA | $5.231 \times 10^{7}$ |
| | Moist | 10 – 40 | 0.63 – 2.69 | 1.13 | 6.9 | $2.599 \times 10^{10}$ | $2.599 \times 10^{9}$ | | | | | $1.093 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 0.20 – 0.63 | 0.33 | 14.6 | $7.590 \times 10^{9}$ | $7.590 \times 10^{9}$ | | | | | $3.191 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.03 – 0.20 | 0.11 | NA | $2.530 \times 10^{9}$ | $2.530 \times 10^{8}$ | | | | | $1.064 \times 10^{6}$ |
| **Russell Branch** Waterbody ID: **026 – 0200** HUC-12: 0307 | High Flows | 0 – 10 | 2.14 – 47.2 | 4.42 | 38.5 | $1.017 \times 10^{11}$ | $1.017 \times 10^{10}$ | NA | NA | NA | NA | $5.446 \times 10^{7}$ |
| | Moist | 10 – 40 | 0.46 – 2.14 | 0.84 | 48.2 | $1.932 \times 10^{10}$ | $1.932 \times 10^{9}$ | | | | | $1.035 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 0.14 – 0.46 | 0.24 | 45.7 | $5.520 \times 10^{9}$ | $5.520 \times 10^{8}$ | | | | | $2.957 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.02 – 0.14 | 0.08 | NA | $1.840 \times 10^{9}$ | $1.840 \times 10^{8}$ | | | | | $9.857 \times 10^{5}$ |
| **Davis Creek** Waterbody ID: **026 – 1000** HUC-12: 0307 | High Flows | 0 – 10 | 20.50 – 513.3 | 41.94 | 56.9 | $9.646 \times 10^{11}$ | $9.646 \times 10^{10}$ | NA | NA | 0 | NA | $5.547 \times 10^{7}$ |
| | Moist | 10 – 40 | 4.54 – 20.50 | 8.24 | 14.8 | $1.895 \times 10^{11}$ | $1.895 \times 10^{10}$ | | | | | $1.090 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 1.41 – 4.54 | 2.38 | 6.2 | $5.474 \times 10^{10}$ | $5.474 \times 10^{9}$ | | | | | $3.148 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.26 – 1.41 | 0.80 | 21.5 | $1.840 \times 10^{10}$ | $1.840 \times 10^{9}$ | | | | | $1.058 \times 10^{6}$ |

**Table E-19 (cont'd) Summary of TMDLs, WLAs, & LAs expressed as daily loads for Impaired Waterbodies in the Powell River Watershed (HUC 06010206)**

| Waterbody Description (TN06010206__) | Hydrologic Condition | | | Flow[a] | PLRG | TMDL | MOS | WLAs | | | | LAs |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| | Flow Regime | PDFE Range | Flow Range | | | | | WWTFs [b] | CS | CAFOs | MS4s | |
| | | [%] | [cfs] | [cfs] | [%] | [CFU/d] | [CFU/d] | [CFU/d] | | | [CFU/d/ac] | [CFU/d/ac] |
| **Davis Creek** Waterbody ID: **026 – 2000** HUC-12: 0307 | High Flows | 0 – 10 | 14.29 – 348.4 | 29.41 | NR | $6.764 \times 10^{11}$ | $6.764 \times 10^{10}$ | NA | NA | 0 | NA | $5.642 \times 10^{7}$ |
| | Moist | 10 – 40 | 3.16 – 14.29 | 5.71 | 33.4 | $1.313 \times 10^{11}$ | $1.313 \times 10^{10}$ | | | | | $1.095 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 0.98 – 3.16 | 1.64 | NR | $3.772 \times 10^{10}$ | $3.772 \times 10^{9}$ | | | | | $3.146 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.19 – 0.98 | 0.55 | NA | $1.265 \times 10^{10}$ | $1.265 \times 10^{9}$ | | | | | $1.055 \times 10^{6}$ |
| **Davis Creek** Waterbody ID: **026 – 3000** HUC-12: 0307 | High Flows | 0 – 10 | 9.87 – 240.5 | 19.67 | 4.0 | $4.524 \times 10^{11}$ | $4.524 \times 10^{10}$ | NA | NA | 0 | NA | $5.462 \times 10^{7}$ |
| | Moist | 10 – 40 | 2.21 – 9.87 | 3.99 | 16.3 | $9.177 \times 10^{10}$ | $9.177 \times 10^{9}$ | | | | | $1.108 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 0.67 – 2.21 | 1.14 | NR | $2.622 \times 10^{10}$ | $2.622 \times 10^{9}$ | | | | | $3.146 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.13 – 0.67 | 0.38 | NA | $8.740 \times 10^{9}$ | $8.740 \times 10^{8}$ | | | | | $1.055 \times 10^{6}$ |
| **Davis Creek** Waterbody ID: **026 – 4000** HUC-12: 0307 | High Flows | 0 – 10 | 2.54 – 61.1 | 5.06 | 22.9 | $1.164 \times 10^{11}$ | $1.164 \times 10^{10}$ | NA | NA | 0 | NA | $5.504 \times 10^{7}$ |
| | Moist | 10 – 40 | 0.57 – 2.54 | 1.02 | 25.5 | $2.346 \times 10^{10}$ | $2.346 \times 10^{9}$ | | | | | $1.110 \times 10^{7}$ |
| | Mid-Range | 40 – 70 | 0.18 – 0.57 | 0.30 | NR | $6.900 \times 10^{9}$ | $6.900 \times 10^{8}$ | | | | | $3.263 \times 10^{6}$ |
| | Low Flows | 70 – 100 | 0.03 – 0.18 | 0.10 | NA | $2.300 \times 10^{9}$ | $2.300 \times 10^{8}$ | | | | | $1.088 \times 10^{6}$ |

Notes: NA = Not Applicable.
NR = No Reduction Required.
PLRG = Percent Load Reduction Goal to achieve TMDL.
CS = Collection Systems
Shaded Flow Zone for each waterbody represents the critical flow zone.

a. Flow applied to TMDL, MOS, and allocation (WLA[MS4] and LA) calculations. Flows represent the midpoint value in the respective hydrologic flow regime.

b. WLAs for WWTFs are expressed as E. coli loads (CFU/day). All current and future WWTFs must meet water quality standards as specified in their NPDES permit.

# APPENDIX F

## Public Notice Announcement

# STATE OF TENNESSEE
# DEPARTMENT OF ENVIRONMENT AND CONSERVATION
# DIVISION OF WATER POLLUTION CONTROL

## PUBLIC NOTICE OF AVAILABILITY OF PROPOSED TOTAL MAXIMUM DAILY LOAD (TMDL) FOR E. COLI IN POWELL RIVER WATERSHED (HUC 06010206), TENNESSEE

Announcement is hereby given of the availability of Tennessee's proposed Total Maximum Daily Load (TMDL) for E. coli in the Powell River watershed, located in eastern Tennessee. Section 303(d) of the Clean Water Act requires states to develop TMDLs for waters on their impaired waters list. TMDLs must determine the allowable pollutant load that the water can assimilate, allocate that load among the various point and nonpoint sources, include a margin of safety, and address seasonality.

**A number of waterbodies in the Powell River watershed are listed on Tennessee's Final 2008 303(d) list as not supporting designated use classifications due, in part, to pasture grazing and septic tanks. The TMDL utilizes Tennessee's general water quality criteria, continuous flow data from a USGS discharge monitoring station located in proximity to the watershed, site specific water quality monitoring data, a calibrated hydrologic model, load duration curves, and an appropriate Margin of Safety (MOS) to establish allowable loadings of pathogens which will result in the reduced in-stream concentrations and attainment of water quality standards. The TMDL requires reductions of pathogen loading on the order of 14-74% in the listed waterbodies.**

**Powell River E. coli TMDL may be downloaded from the Department of Environment and Conservation website:**

**http://www.state.tn.us/environment/wpc/tmdl/**

Technical questions regarding this TMDL should be directed to the following members of the Division of Water Pollution Control staff:

Vicki S. Steed, P.E., Watershed Management Section
Telephone: 615-532-0707

Sherry H. Wang, Ph.D., Watershed Management Section
Telephone: 615-532-0656

Persons wishing to comment on the proposed TMDLs are invited to submit their comments in writing no later than September 15, 2008 to:

Division of Water Pollution Control
Watershed Management Section
7th Floor, L & C Annex
401 Church Street
Nashville, TN 37243-1534

All comments received prior to that date will be considered when revising the TMDL for final submittal to the U.S. Environmental Protection Agency.

The TMDL and supporting information are on file at the Division of Water Pollution Control, 6th Floor, L & C Annex, 401 Church Street, Nashville, Tennessee. They may be inspected during normal office hours. Copies of the information on file are available on request.